

SESSION 1 – THE PAST:
Verifying Computer Analysis Results with
Hand Calculations



SAM RUBENZER, PE, SE

- Founded FORSE Consulting in 2010
- Assists structural engineers on a wide variety of designs with an assortment of structural engineering design software



FORSE

- Many years of experience as licensed engineers
- FORSE has worked hard to learn each of the software programs used by SEs and created many presentations comparing attributes of different software tools
- Worked as consultants with software companies teaching others about SE software



Abstract

- Structural Engineers are relying more and more on structural engineering software for analysis and design.
- Understanding the different options available for modeling is paramount in ensuring the best model is created to imitate reality and give engineers the best possible design.
- This presentation reviews various hand calculation methods for verifying the loads defined on models, and verifying the analysis results.
- Lastly, we will verify the design checks made for members within the model. It is easy to assume that all structural engineering software solves engineering problems correctly. Unfortunately, there can be errors from programming and user mistakes. Engineers must have a good understanding apart from software to spot these errors.



A Word About Software

- Structural engineers rely on finite elements models for analysis and design
- Understanding the different options available for modeling is paramount
- Best model is created to imitate reality and give engineers the best possible design



Is Your Software Model a Good Representation of Reality?



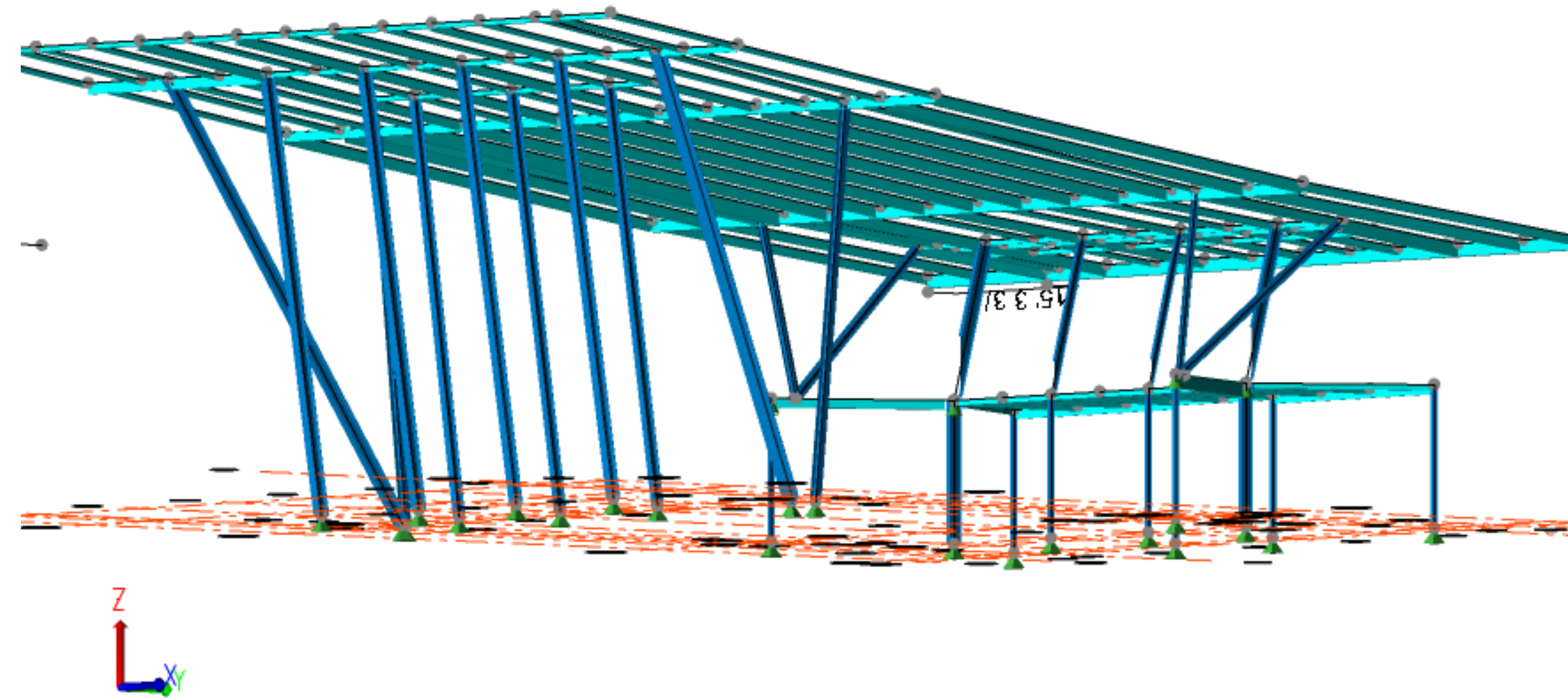
Structural Engineering Software

- Software is continuously changing our ability to do many things in our lives, personal and professional
- This is no different with structural engineering. Software will make you a better engineer as long as you use the software as a tool, and don't become an "operator"
- Never let the software think for you, only let it think faster
- Never let the software decide for you. Period.



Structural Engineering Software

- Never assume the software is doing anything correctly
- Never assume the software is making the same decisions you would make
- Software programs are tools, you are the engineer, never forget that



Structural Engineering Software

- **Never assume the software is correct**, or as you would have done it “by hand”
- Examples
 - certain programs will distribute load one-way
 - regardless of the span aspect ratio, even 100:1
 - automatic features are by far the most dangerous
 - settings aren't apparent when using software, in the manual
 - **default settings** are dangerous
 - **create a false sense of a “standard”**



Structural Engineering Software: “Do You Agree with the Programmer?”

- Structural engineers also rely on:
 - Education, experience, and guidance from the code
 - Your good engineering judgment is still invaluable
- **When programmers develop software for us to use, they are relying on codes and their own judgment**
- You will find that your judgment isn't always in agreement with another structural engineer's
 - Don't use a feature you don't agree with
 - Don't assume other users agree ...
...so it must be OK?



Need to Know What We Don't Know...



Stated Learning Objectives

- Verify loads applied to models
- Verify analysis results
- Verify design checks

**IN ORDER TO VERIFY, YOU NEED TO KNOW WHAT
THE PROGRAM IS DOING**



Let's Start with Philosophy

“Those that wish to succeed must ask the right preliminary questions”

- Aristotle



Verify Loads Applied to Models

- Manually determine loads on structure
- Approximate distribution
- Know software load generator capabilities
- Review applied loads after load generator application



Verify Loads Applied to Models



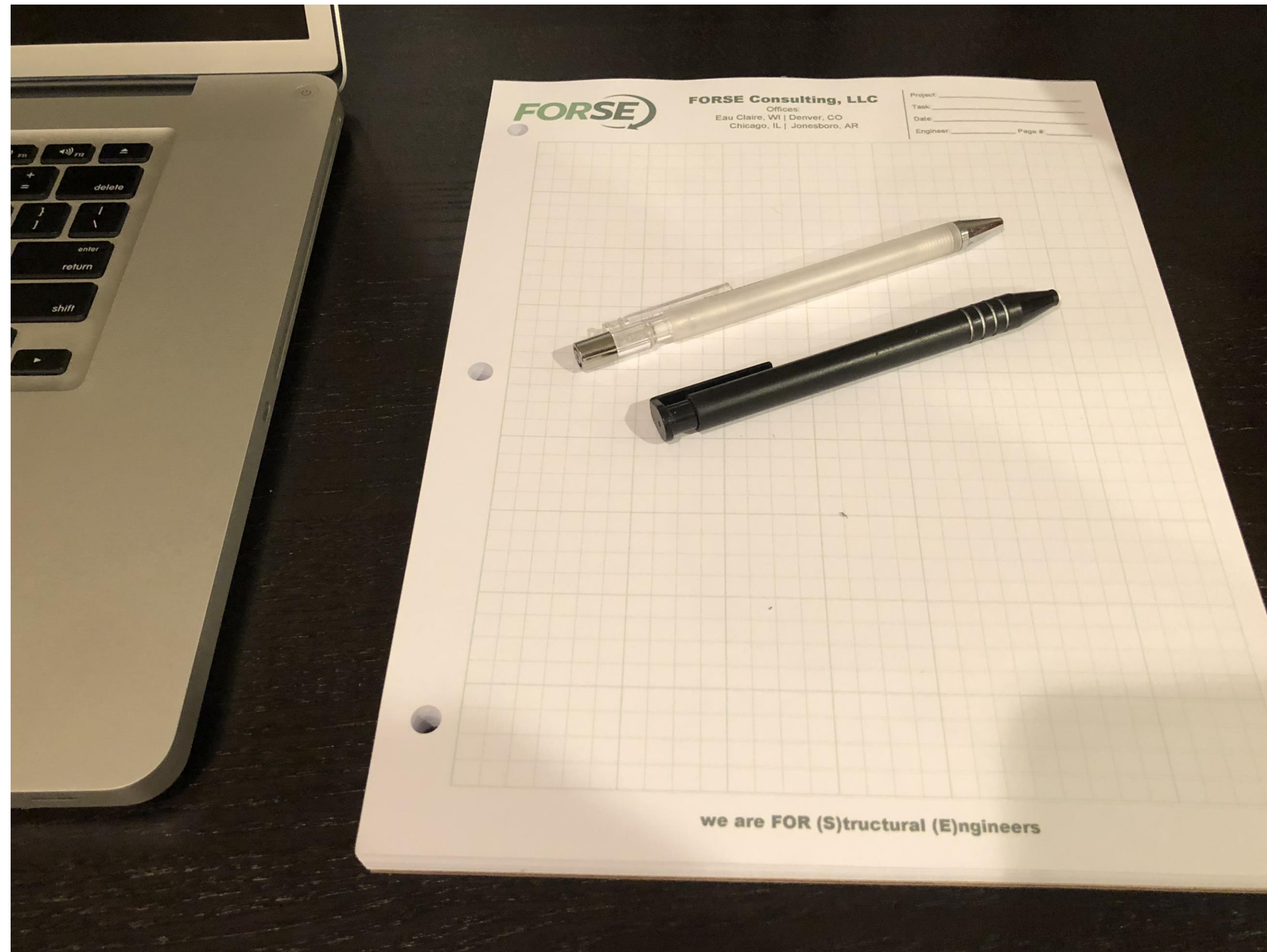
Verify Loads Applied to Models

- Manually determine loads on structure
- Approximate distribution
- HAND CALCS or SPREADSHEETS



Verify Loads Applied to Models

Approximate Load and Distribution



Verify Loads Applied to Models

Approximate Load and Distribution

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 Project: Bliss Checked By: Date:

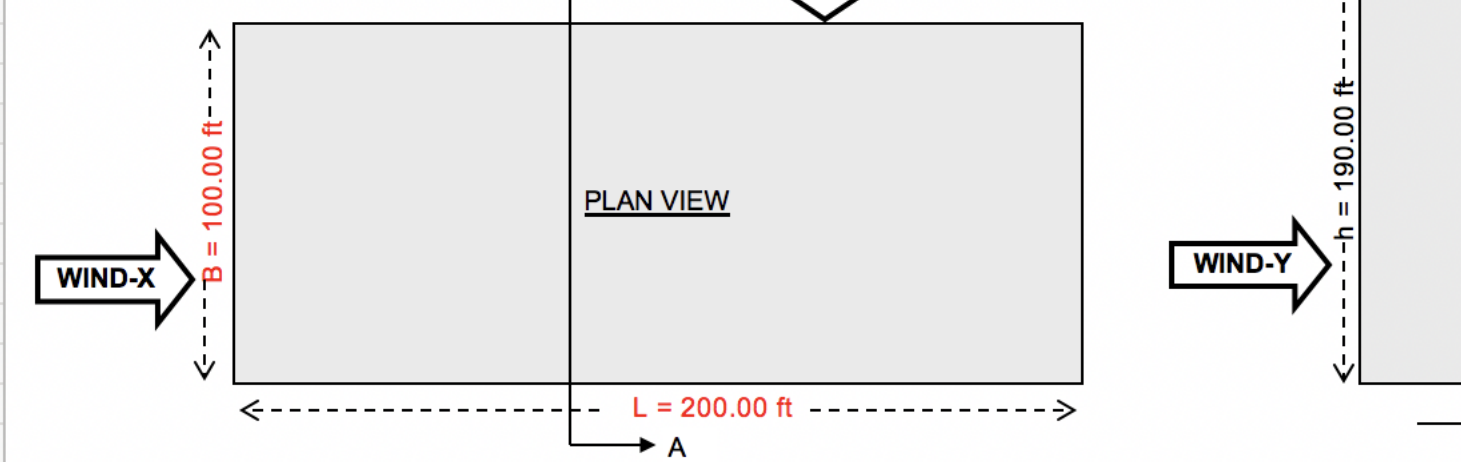
FORSE Project#: xx Designed By: SMR Date: 6/17/18
 Project: xx Checked By: Date:

SECTION 6.0 WIND LOADS ASCE 7-05 Minimum Design Loads for Buildings and Other Structures

Section 6.5 METHOD 2 - ANALYTICAL PROCEDURE
 6.5.1 Scope 6.5.2 Limitations 6.5.3 Design Procedure Guidelines for use

Multi-Story - Flat Roof Building

Number of Stories = 18
 Story Height = 10.00 ft
 Parapet Height = 10



Step 1. Determine Wind Speed, V and Wind Directionality Factor, K_d

Section 6.5.4 Basic Wind Speed
 V = 145 mph [Figure 6-1]
 Section 6.5.4.4 Wind Directionality Factor, K_d
 Buildings - Main Wind force Resisting S_w
 K_d = 0.85 [Table 6-4]

Step 2. Determine an Importance Factor

Section 6.5.5 Importance Factor, I
 Importance Category ASCE IV [Table 1-1]
 Hurricane Prone Region No
 I = 1.15

Step 3. Determine an exposure category or categories and velocity pressure exposure coefficient

Section 6.5.6 Exposure Categories
 Exposure D [See Section 6.5.3.2 on pg. 29 for Ex]
 Mean roof height, h = 190.0 ft → K_z = 1.602

Main Wind Force Resisting System, MWFRS
 for MWFRS of buildings of all heights

z [ft]	K _z
190.00	1.602
178.13	1.584
166.25	1.565
154.38	1.545

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Seismic Loads - IBC 2000

1. Determine mapped spectral response acceleration values

1615 EARTHQUAKE LOADS- SITE GROUND MOTION
 S_S = 0.060 g
 S₁ = 0.040 g

2. Determine Site Class (A-F)

3. Determine F_a & F_v
 SOIL SITE CLASS ? D [Table 1615.1.1]
 F_a = 1.60 [Table 1615.1.2(1)]
 F_v = 2.40 [Table 1615.1.2(2)]

4. Determine adjusted spectral response acceleration values

1615.1.2 SITE COEFFICIENTS AND ADJUSTED MCE SPECTRAL RESPONSE ACCELERATION PARAMETERS
 S_{MS} = F_a * S_S = 0.096 g [Equation 16-16]
 S_{M1} = F_v * S₁ = 0.096 g [Equation 16-17]

5. Determine design spectral response acceleration values

1615.1.3 DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS (5% damped design spectral response acceleration)
 S_{DS} = ²/3 * S_{MS} = 0.064 g [Equation 16-18]
 S_{D1} = ²/3 * S_{M1} = 0.064 g [Equation 16-19]

6. Determine Seismic Use Group of Bldg

SEISMIC USE GROUP ? III [SECTION 1616.2]

7. Determine Seismic Design Category

1616.3 SEISMIC DESIGN CATEGORY
 SEISMIC DESIGN CATEGORY as determined by S_{DS} → A
 SEISMIC DESIGN CATEGORY as determined by S_{D1} → A } **USE → A** [Table 1616.3(1)]

8. Determine Minimum Allowable Analysis Procedure for Seismic Design

Number of Stories, N = 1
 Minimum Story Height = 24.00 ft
 Building Height, h_n = 24.0 ft

Light-framed construction ? yes
LIGHT-FRAME CONSTRUCTION: A method of construction where the structural assemblies (e.g., walls, floors, ceilings, and roofs) are primarily formed by a system of repetitive wood or cold-formed steel framing members or subassemblies of these members (e.g., trusses).

Regular Structure ? yes

Flexible Diaphragm at Every Level? yes [Defined in SECTION 1602.1]

Low Loads

Ground snow load, p_g
 Snow Loads, p_g [Figure 7-1 Ground Snow Loads]

Roof snow load, p_r

of Snow Loads, p_r A flat roof is a roof with the slope equal to or less than 5 degrees or 1 inch per foot
 Terrain: B
 Exposure: Partially Exposed
 Thermal Condition: 1
 Imp. Category: II
 p_q = 21.0 psf [EQ 7 - 1]
 20.0 psf [(p_q*1) if p_q is less than or equal to 20psf; p_r(min) = (20*1) if p_g exceeds 20psf]
 21.0 psf [See Section 7.10 below for p_{r,s} value]

slope C_s

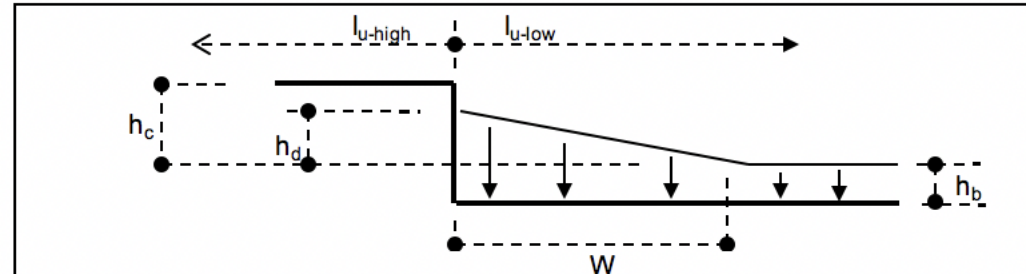
Minimum Values of p_r for Low-Sloped Roofs
 Values shall apply for hip and monoslope roofs with slopes of less than 15 degrees

Partial Snow Loading Skip load areas loaded by snow load to obtain a maximum stress and/or deflection values

Wind Drifts

Wind Drifts on Lower Roofs (Aerodynamic Shade)

Wind Drifts are not required
 30 pcf = 17.9 pcf
 21.0 psf
 1.173 ft



Wind Drifts - low roof elevation) - h_b
 h_d = 0.43 * ³/√(l_u) * ⁴/√(p_g + 10) - 1.5
 h_c = clear height
 W = width of the snow drift
 h_{d-leeward} = drift height when wind is from the high roof (leeward direction)
 h_{d-windward} = drift height when wind is from the low roof (windward direction)
 h_b = base snow depth

Unbalanced snow loading (not applicable for low-rise (flat) roofs)

Rain-on-snow surcharge loads

Rain-on-Snow Surcharge Load
 5 psf
 5.0psf but greater than 0psf, a 5psf rain-on-snow surcharge load shall be applied for roofs with a slope
 on a flat roof, if minimum values are used for p_r, a reduction of [p_r(min) - p_r(EQ 7-1)] up to 5 psf shall be applied to p_{r,s}

Length, l _u	Determine the height of the drift	Resulting Snow Drift Loads	Notes
1.0 ft	h _{d-leeward} = 1.66 ft <--controls	P _{snow drift} = 50.8 psf	* - W exceeds length of the low roof, snow drift load is truncated, not equal to 0

Verify Loads - Gravity Load Generators?

- Several programs distribute load without checking span
 - How far can load be distributed?
- Dead
 - Self weight based on members modeled - don't forget about the elements not modeled, often referred to as super imposed dead load
- Live
 - Live load keyed to ASCE table based on floor usage
 - reducible or not reducible is not the software's decision to make
- Snow or Roof Live load
 - No automated snow drift generators or ponding load generators on the market



Verify Loads - General

- when using a structural analysis and design software package, there is a tendency to assume that the program is correctly generating the loads
- software programmers are very good at interpreting and implementing the codes
 - can't automate every condition that exists in our complex architectural world
- to understand this completely as it pertains to your projects
 - best to know where software programmers get the loads for the load generators.
 - specifically, what sections of the code are used for load generators in common software packages.



Verify Loads - Questions

- When an engineer chooses to generate wind loads, what sections of the code are considered? For example:
 - How many directions is the load applied?
 - Can enclosed, partially enclosed, and open structures be considered?
- When generating seismic loads what code provisions are considered? For example:
 - Using approximate building period or calculated period?
 - Is accidental torsion checked and provisions applied?



Wind Based on ASCE 7

- ENVELOPE PROCEDURE
- DIRECTIONAL PROCEDURE
 - portions of this procedure are generally used by software to generate wind loads
- WIND TUNNEL PROCEDURE
- Versions
 - 7-02
 - 7-05
 - 7-10 (major change)
 - 7-16



Verify Loads - Quick Facts

- ASCE 7-10
 - pages dedicated to Wind
 - 130
 - pages used for most wind load generators
 - Estimate 10-20, varies depending on software
- so when a software indicates ASCE 7-10 is implemented, be sure you know what that means, what's included, and perhaps more importantly, what's excluded!

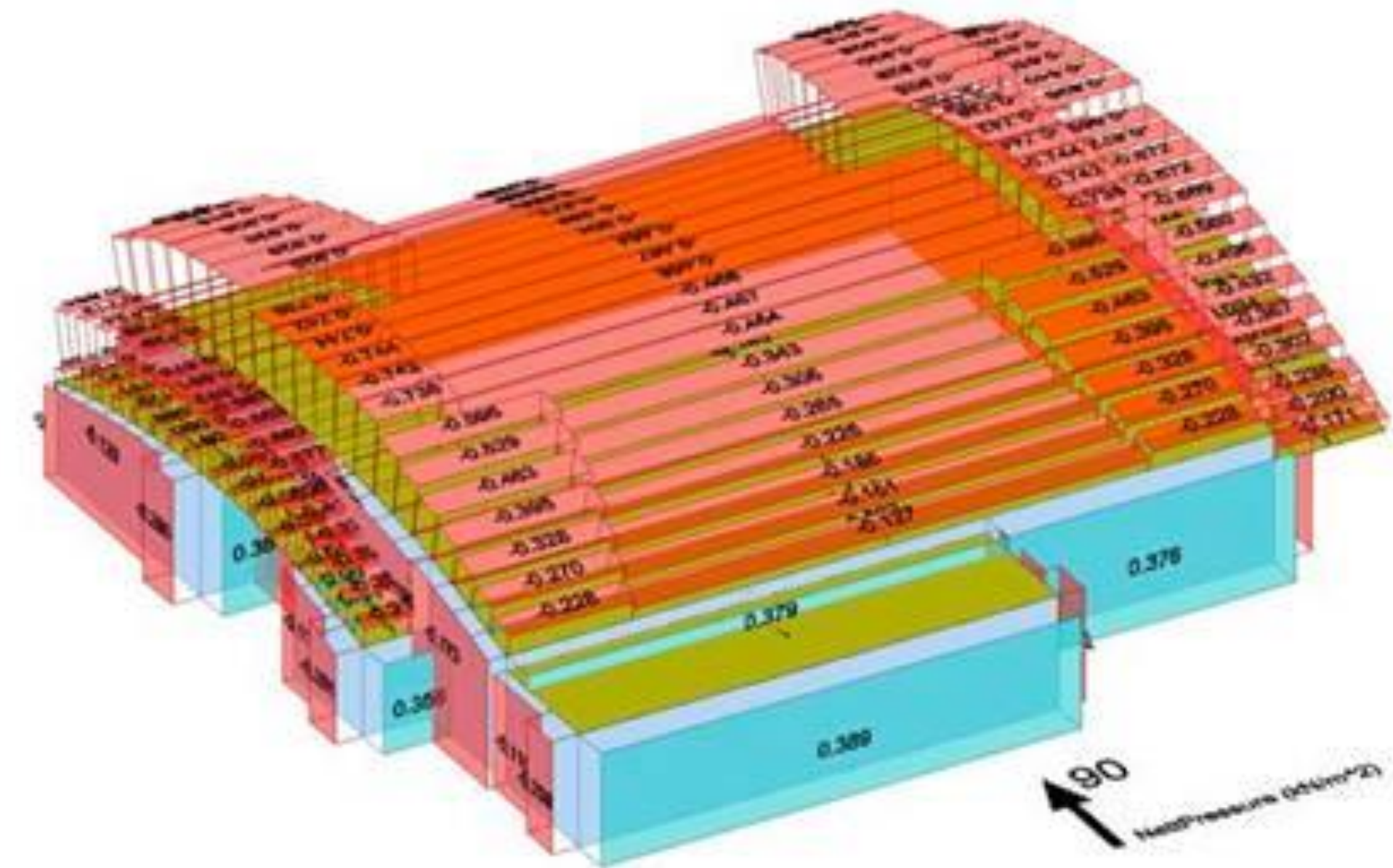


Software Options and Examples



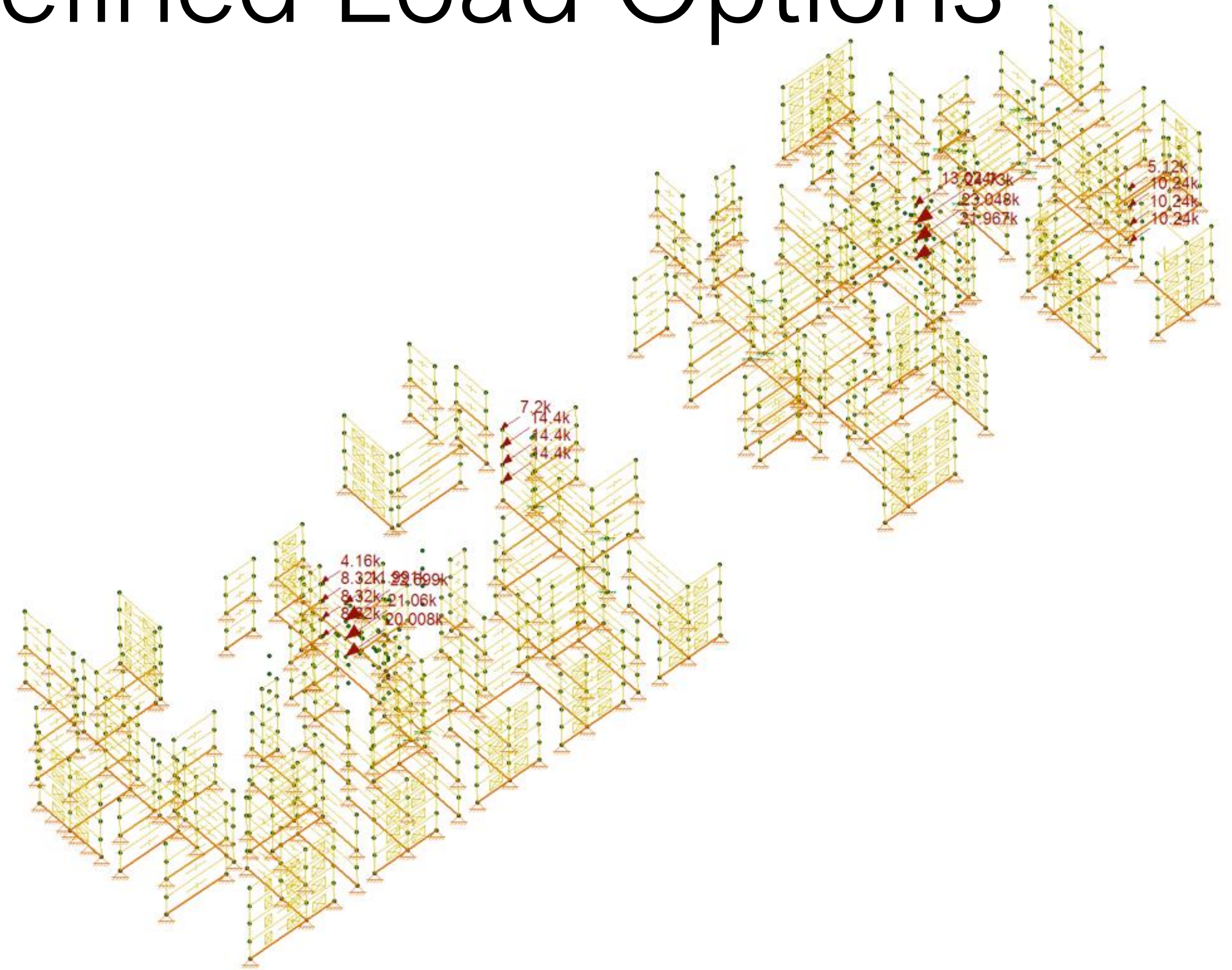
Software Options and Examples

- Auto exposure edges?
 - Determined from defined deck/slab edges
 - Allow Modifications?
- User defined exposure areas?
 - Can user manually define wind exposure areas and distribution?
- Combine with user defined loads?
- Parapets?



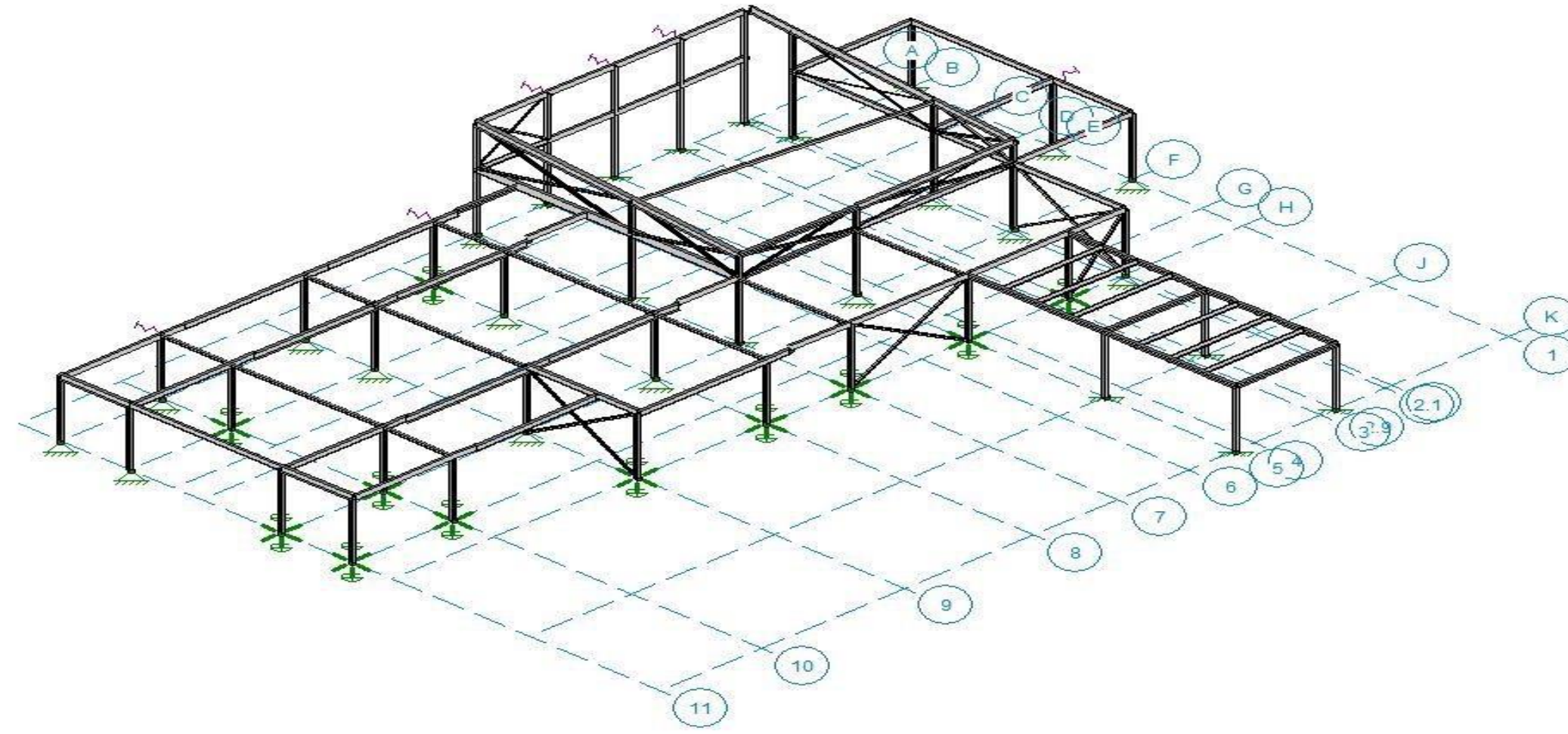
Verify Loads - User Defined Load Options

- What do you do when the load generator is close, but needs supplemental loads to be added
- Can supplemental loads be added to generated loads?



Wind	Auto exposure edges? Allow Modifications?	User defined exposure areas	Combine with user defined loads?	Parapets
RISA 3D	yes no		yes	
RSS (FRAME) RAM Elements	yes yes			yes
ETABS	yes yes	lateral walls	yes	yes
SCIA		load panels	yes	yes
TEKLA Structural Designer		wall panels	yes	yes
IES VisualAnalysis		areas	yes	yes



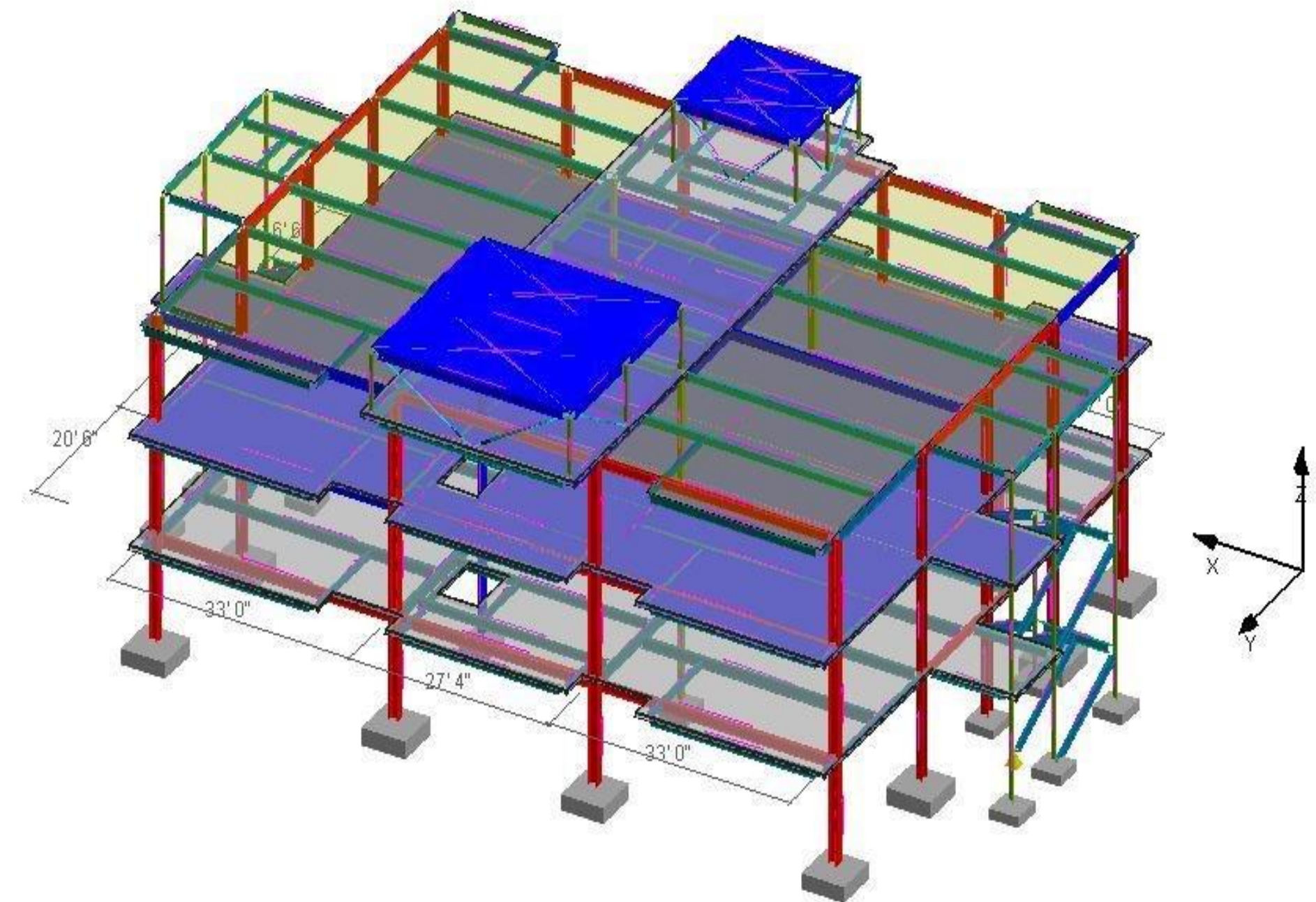


QA/QC for Loading



QA/QC

- Peer review of model is essential
- Loads in = Loads out
- Does resulting base shear = applied lateral load?
- Wind Load Code Check:
 - If factored wind loads are applied per ASCE 7-10, confirm LRFD design is applied

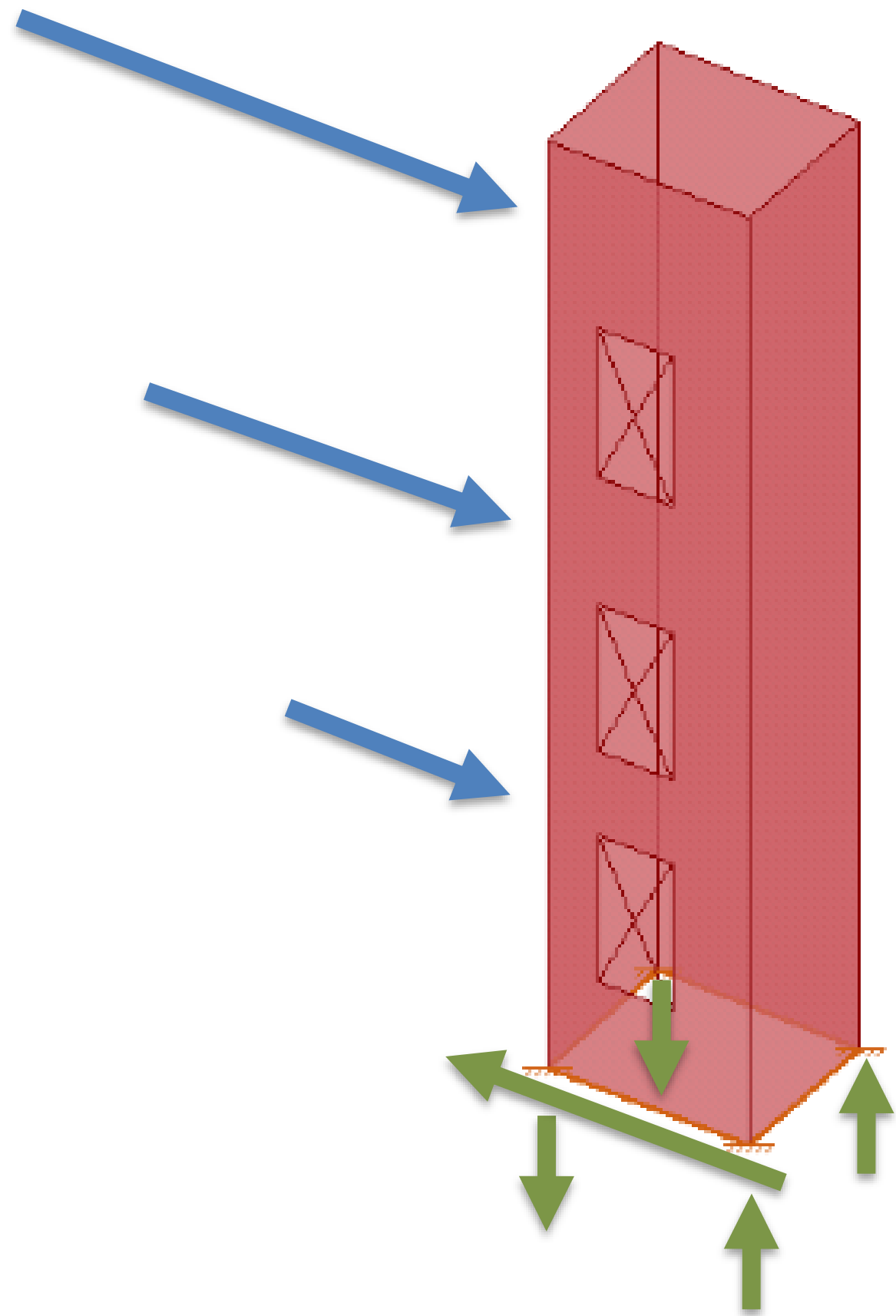


Verify analysis results

- Start with simple models to approximate results
 - Simple micro models
 - Simple macro models
- Work in complex elements to overall model
- Verify final model matches behavior of simple model
- Understand software capability/limitations of analysis



Estimate Behavior Before Hitting Analyze

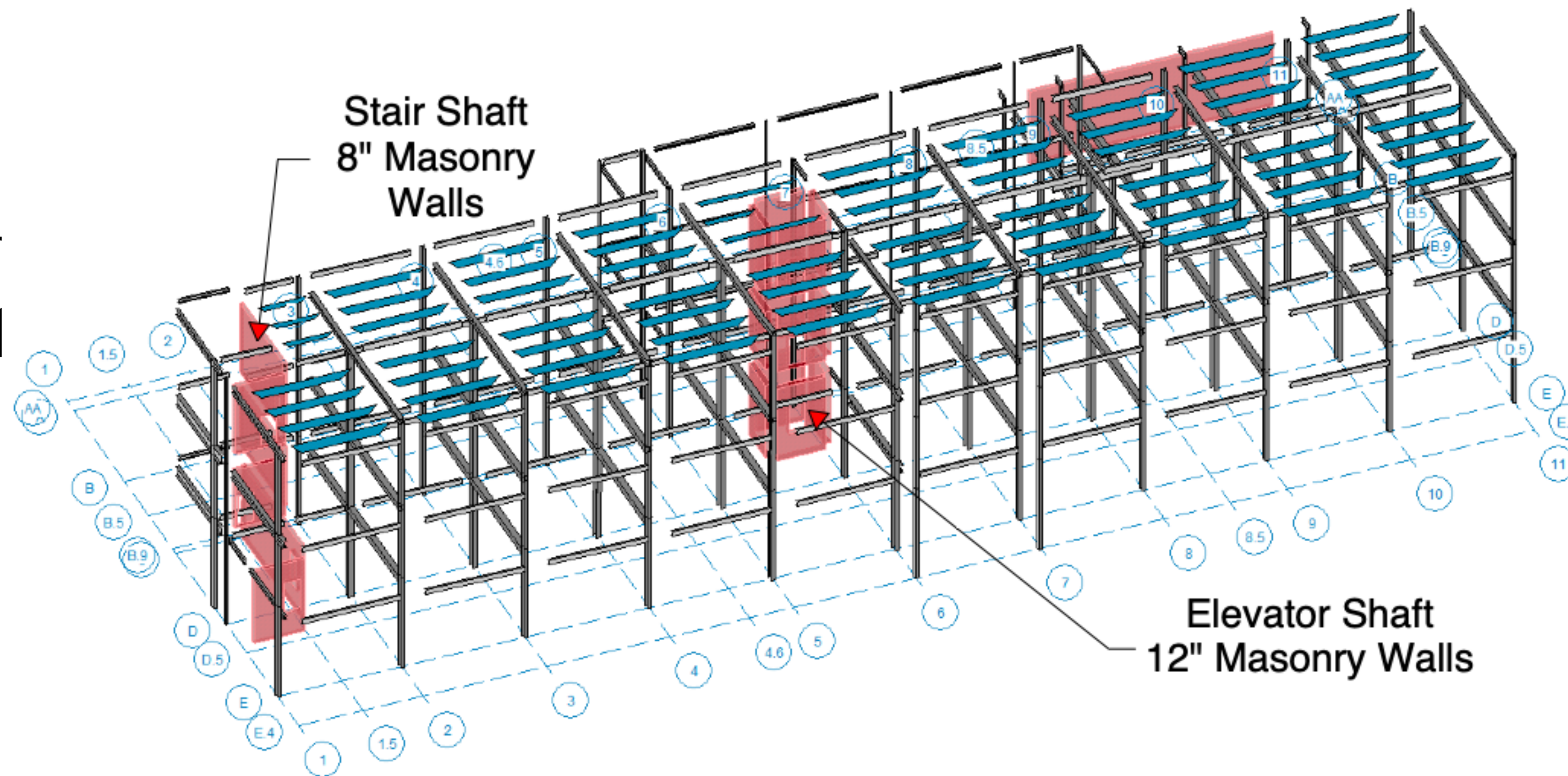


- Estimate load
- Determine Shear for group
- Determine corner up/down reactions as estimate



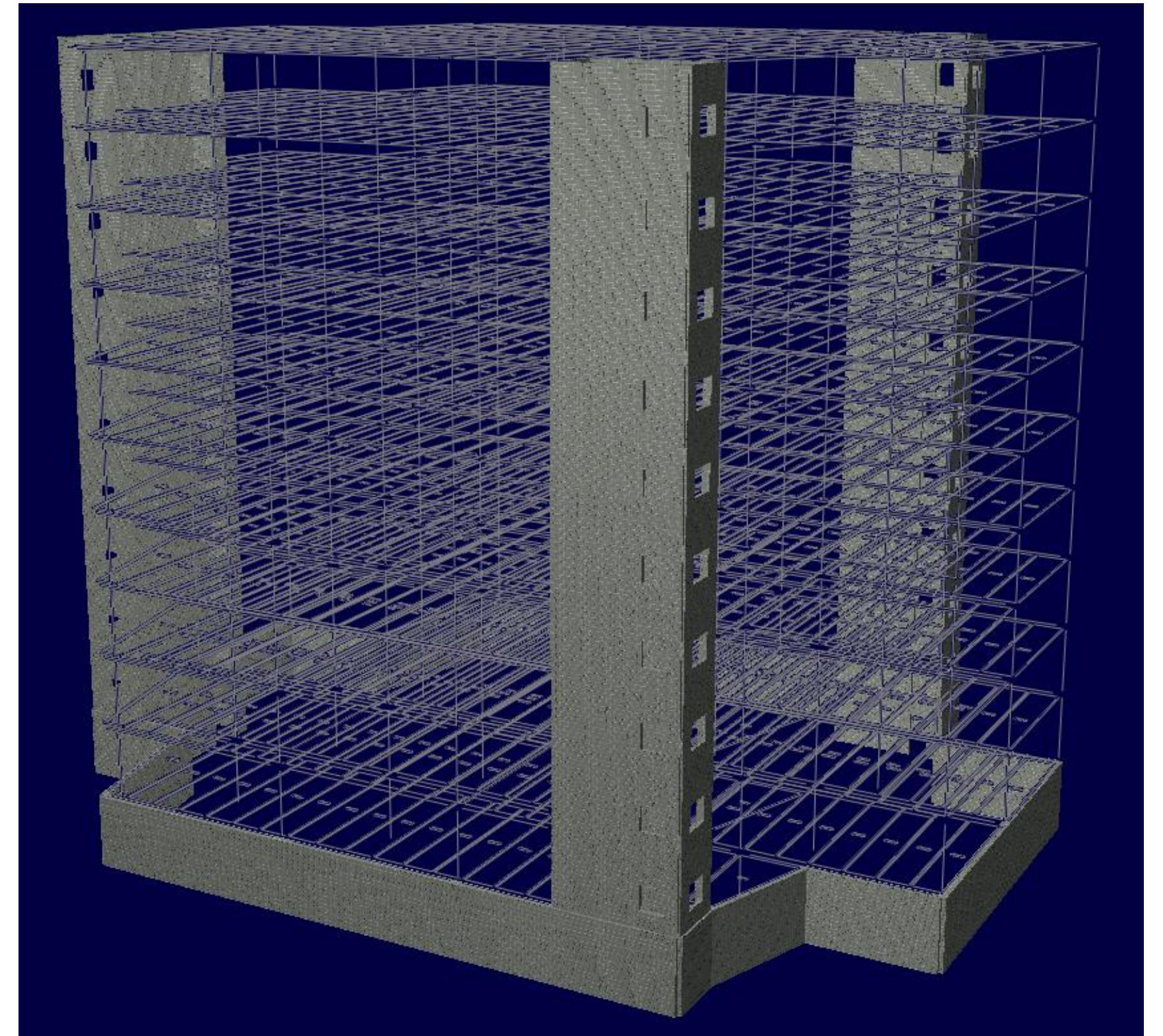
Estimate Behavior Before Hitting Analyze

- Simple to complex
- Lose the ability to do this wr we import complex models f BIM models
- Counter intuitive behavior?
- Real or not real???

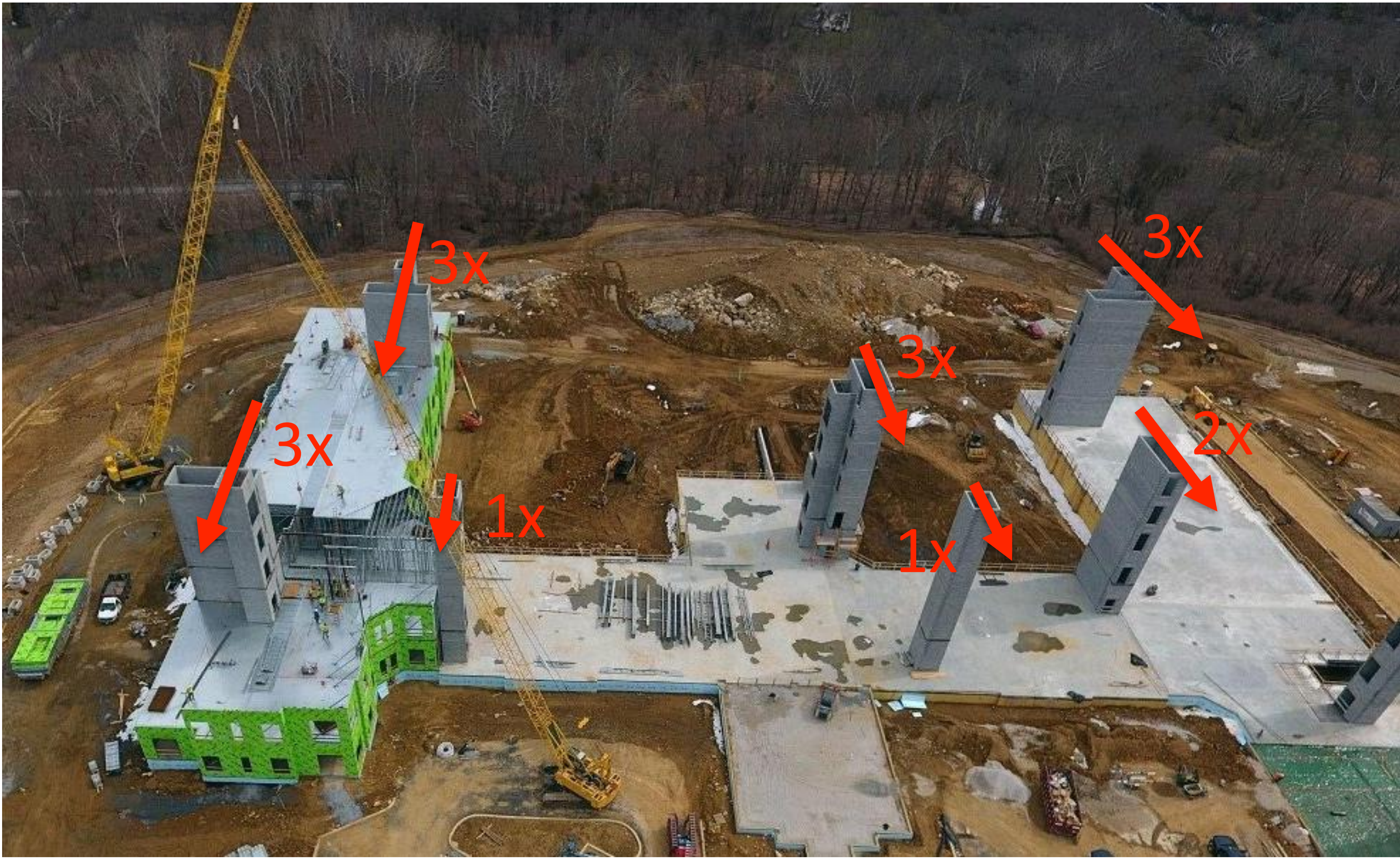


What Happens at First Floor?

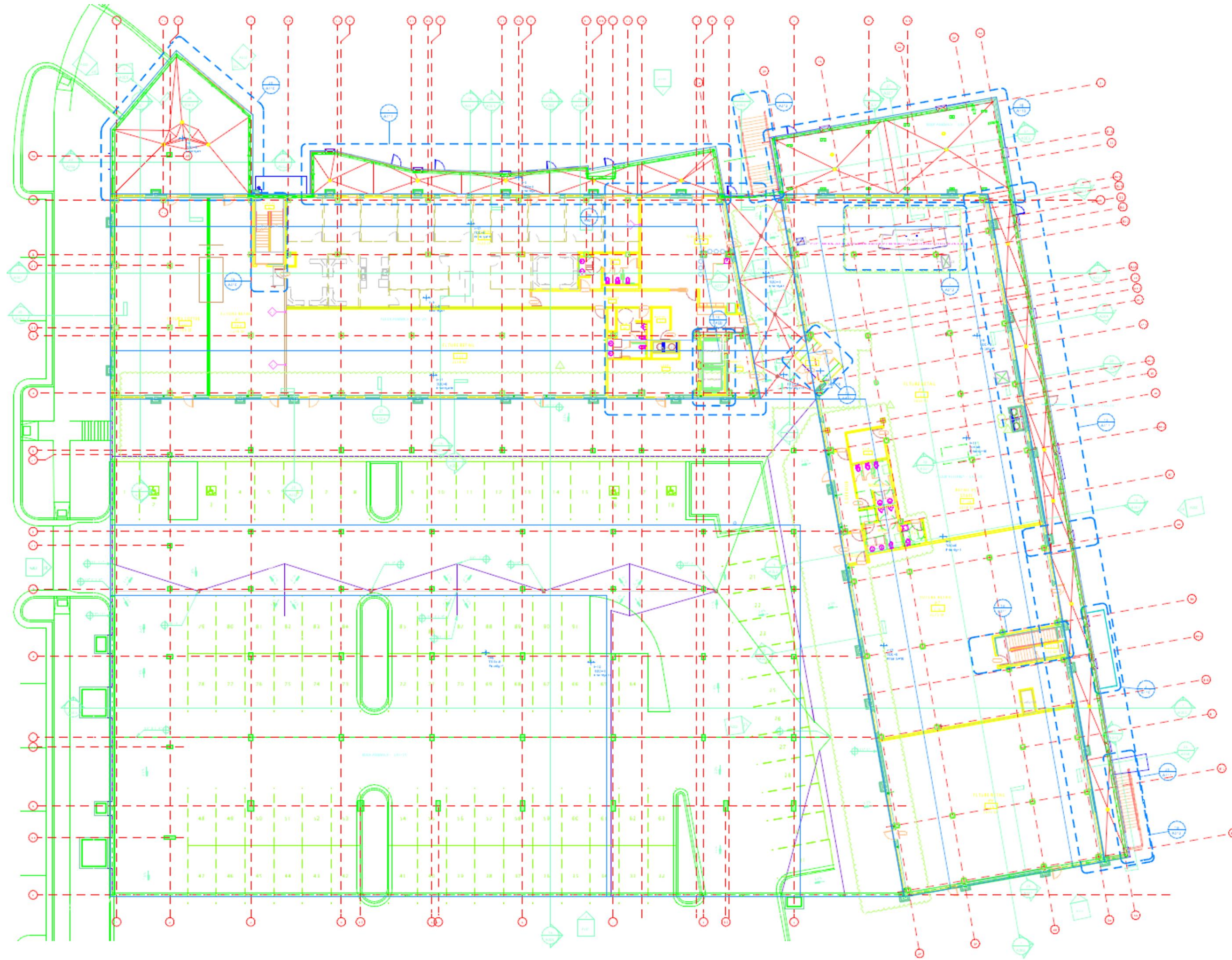
- Can load really reverse in towers?
 - created by rigid diaphragms
- More realistic with Semi-rigid
 - still need to check ability to get load out of wall groups, into diaphragm, then into new walls at foundation



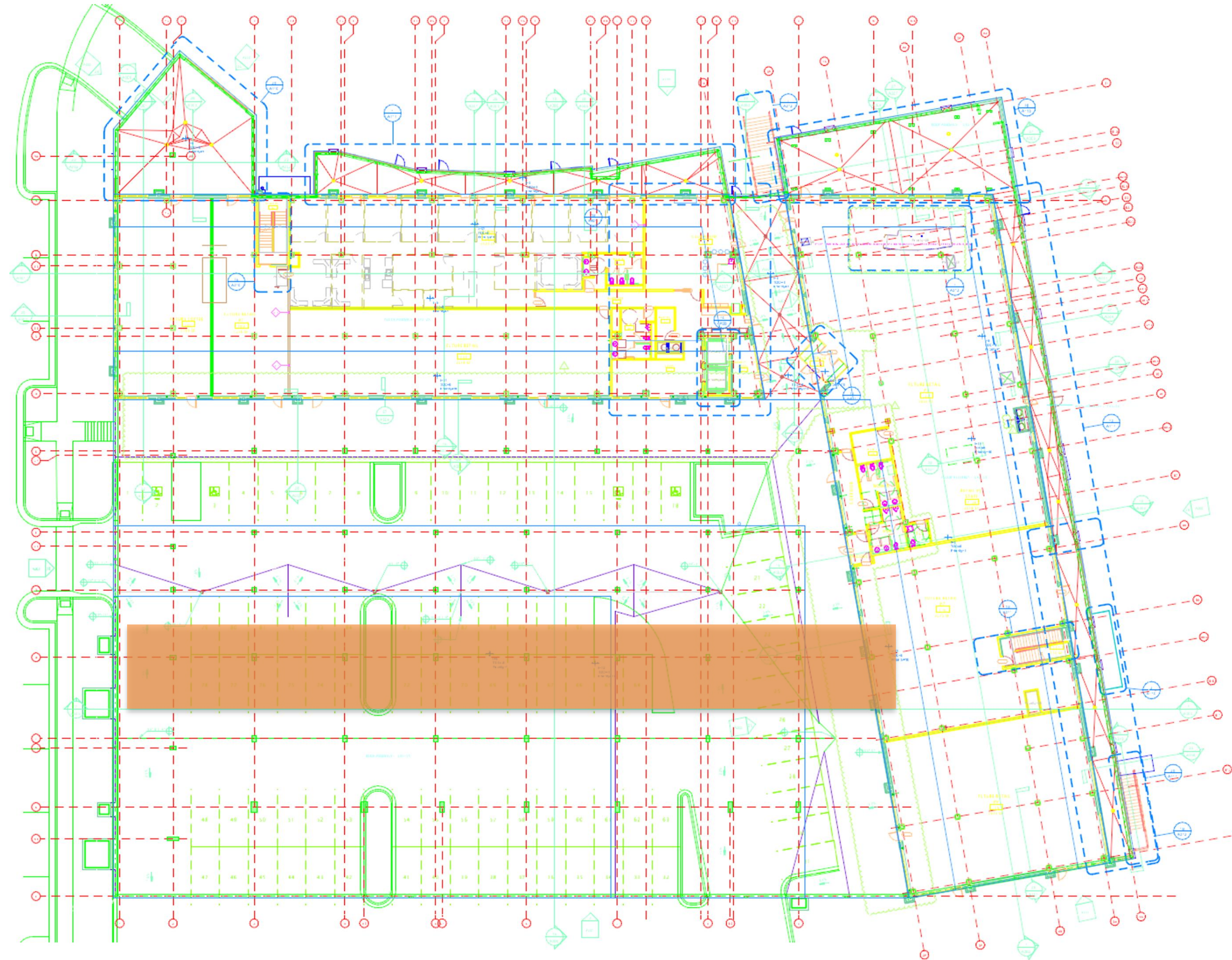
Estimate Load Distribution



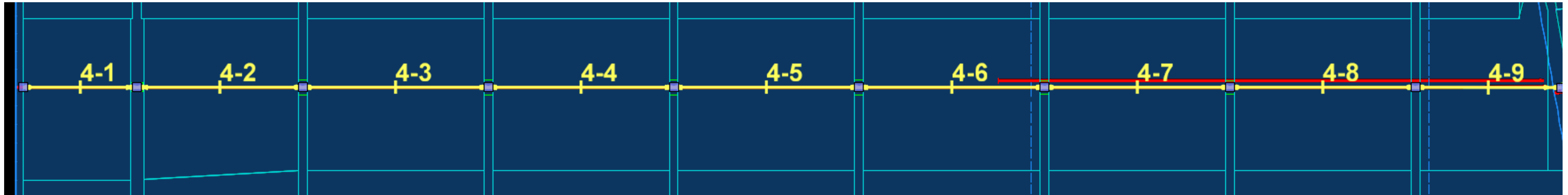
Before We Take on This...



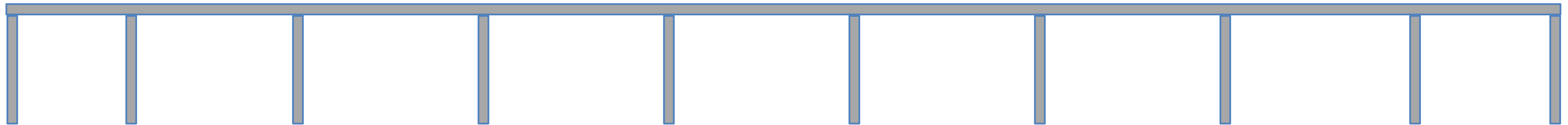
Work on Understanding Individual Area



Work on Understanding Individual Area



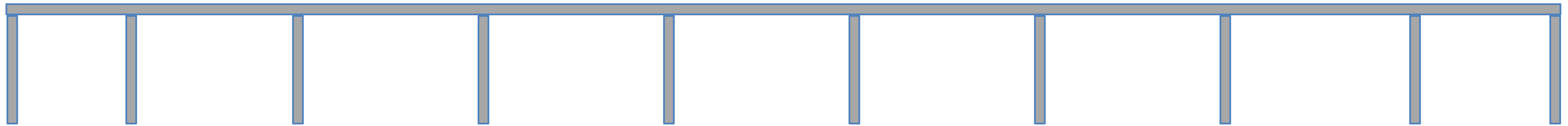
PLAN



SECTION



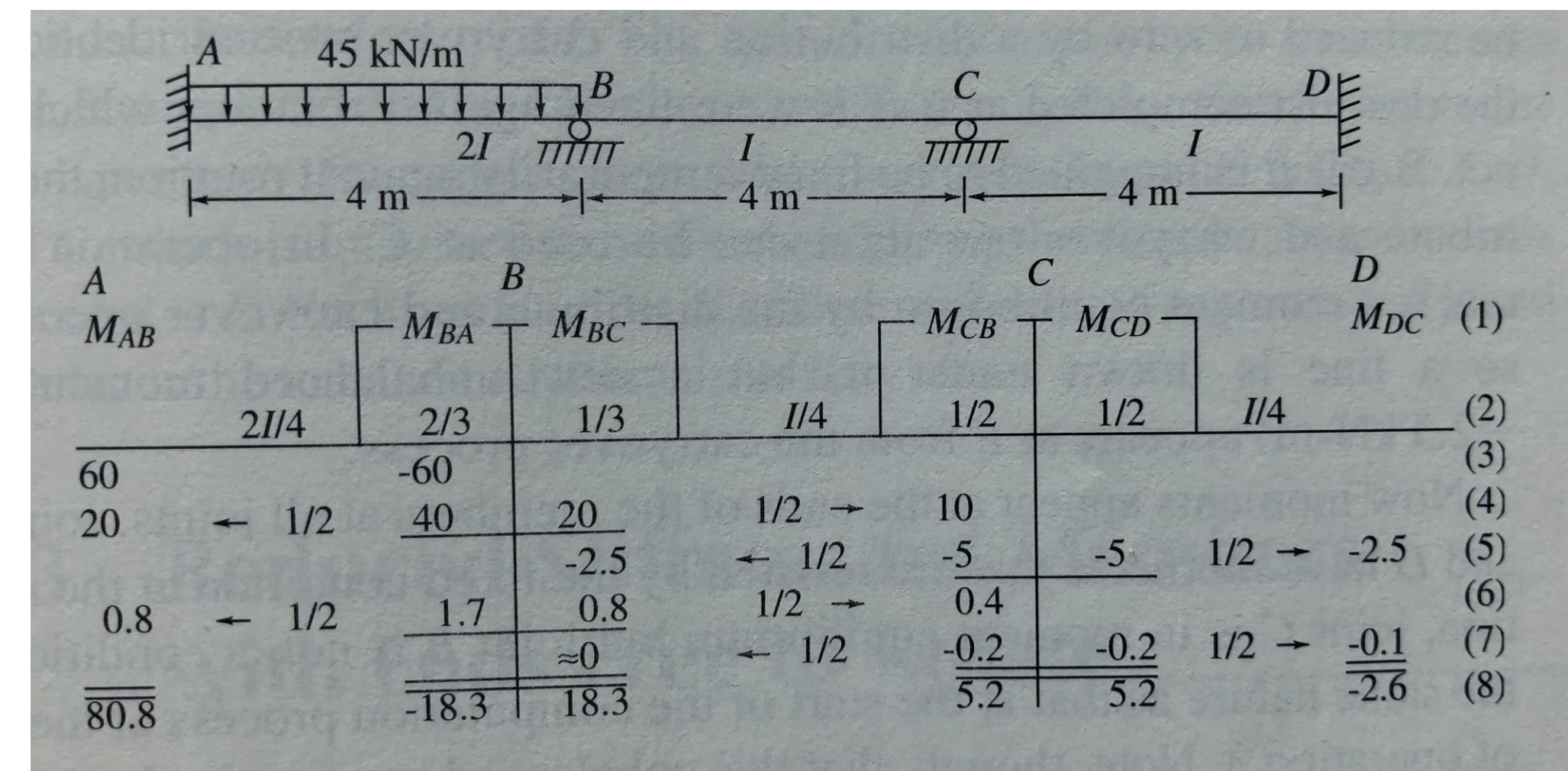
Use hand calcs to estimate



SECTION

Indeterminate Structures

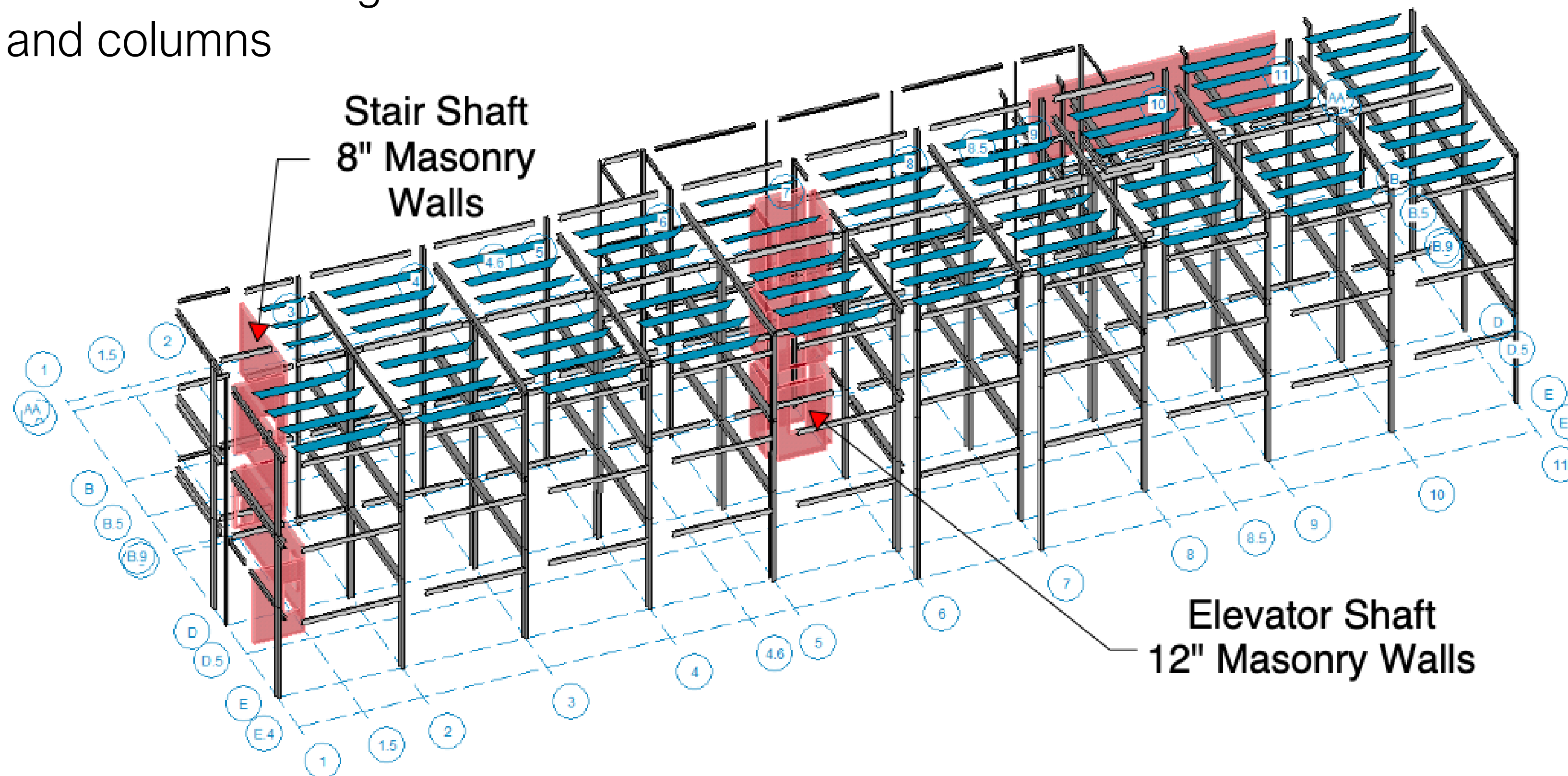
- Slope Deflection
- Moment Distribution



Moment Distribution Example from "ANALYSIS AND BEHAVIOR OF STRUCTURES"

Verify All Elements are Modeled

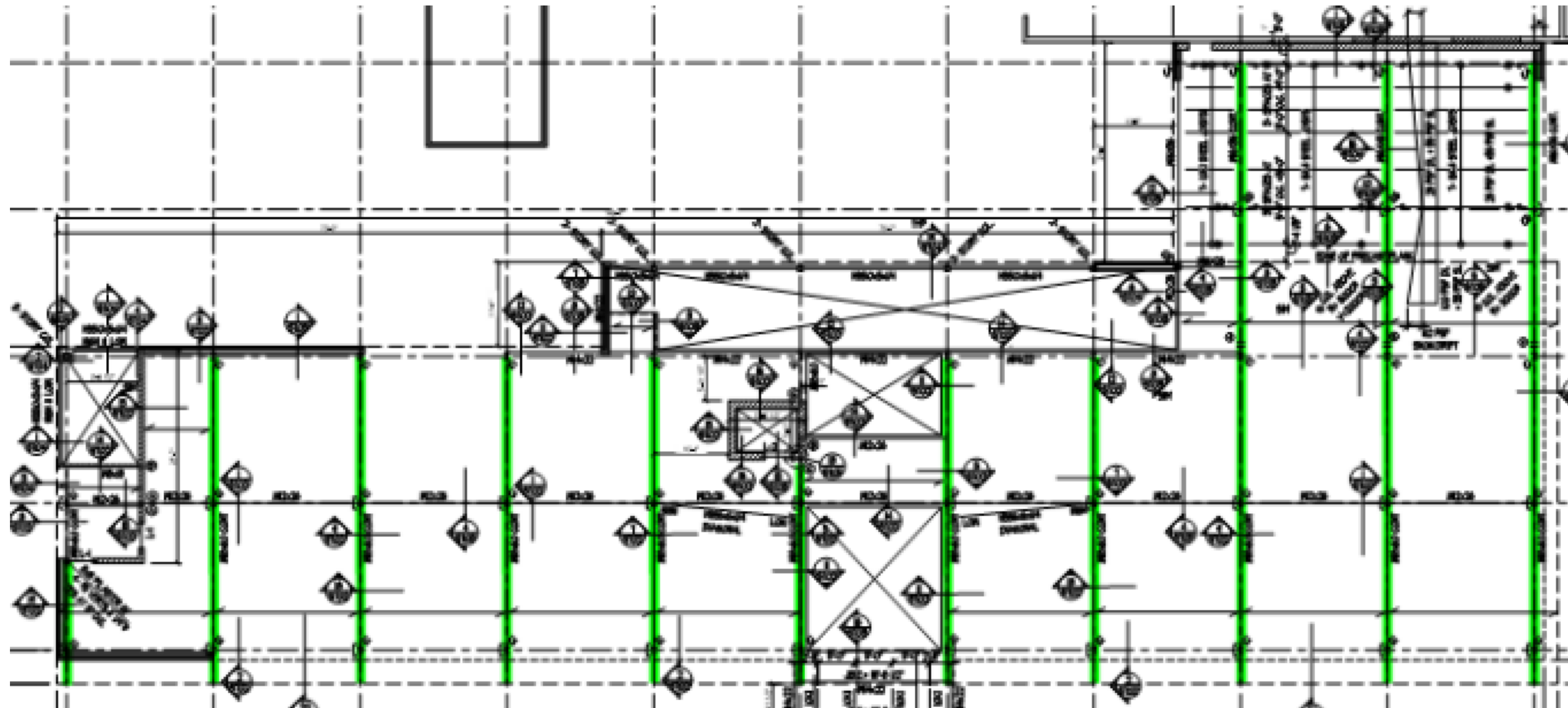
- 3 story structure
- Masonry stair and elevator shaft walls
- Steel floor framing and columns
- Semi-rigid diaphragms at Level 1 and 2
- Flexible diaphragm at roof



Verify All Elements are Modeled

Steel Lateral System

- Steel beams, roof joists, and columns
- 11 Moment Frames in the N-S direction

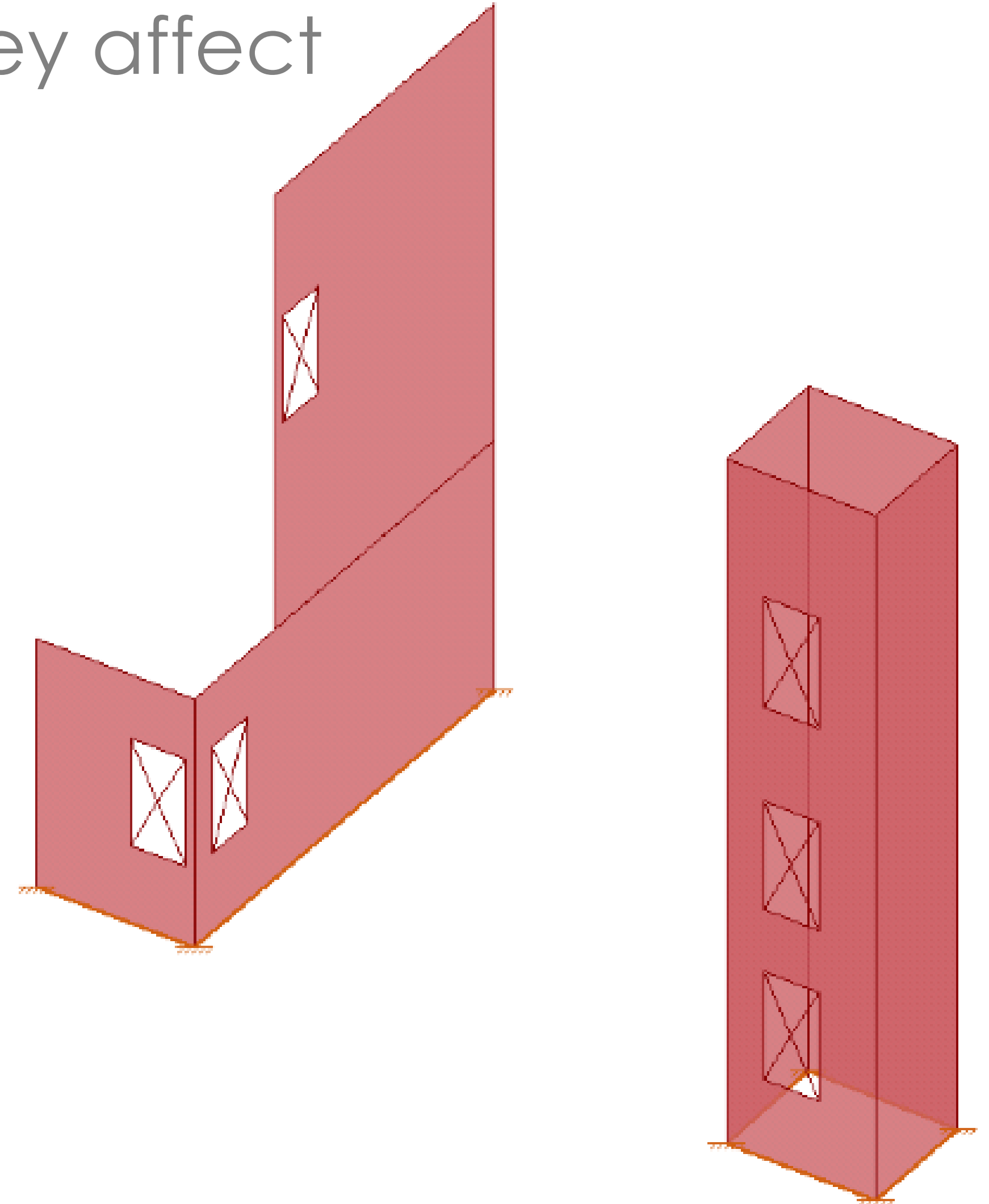


Verify All Elements are Modeled

Forgotten Elements - how will they affect behavior?

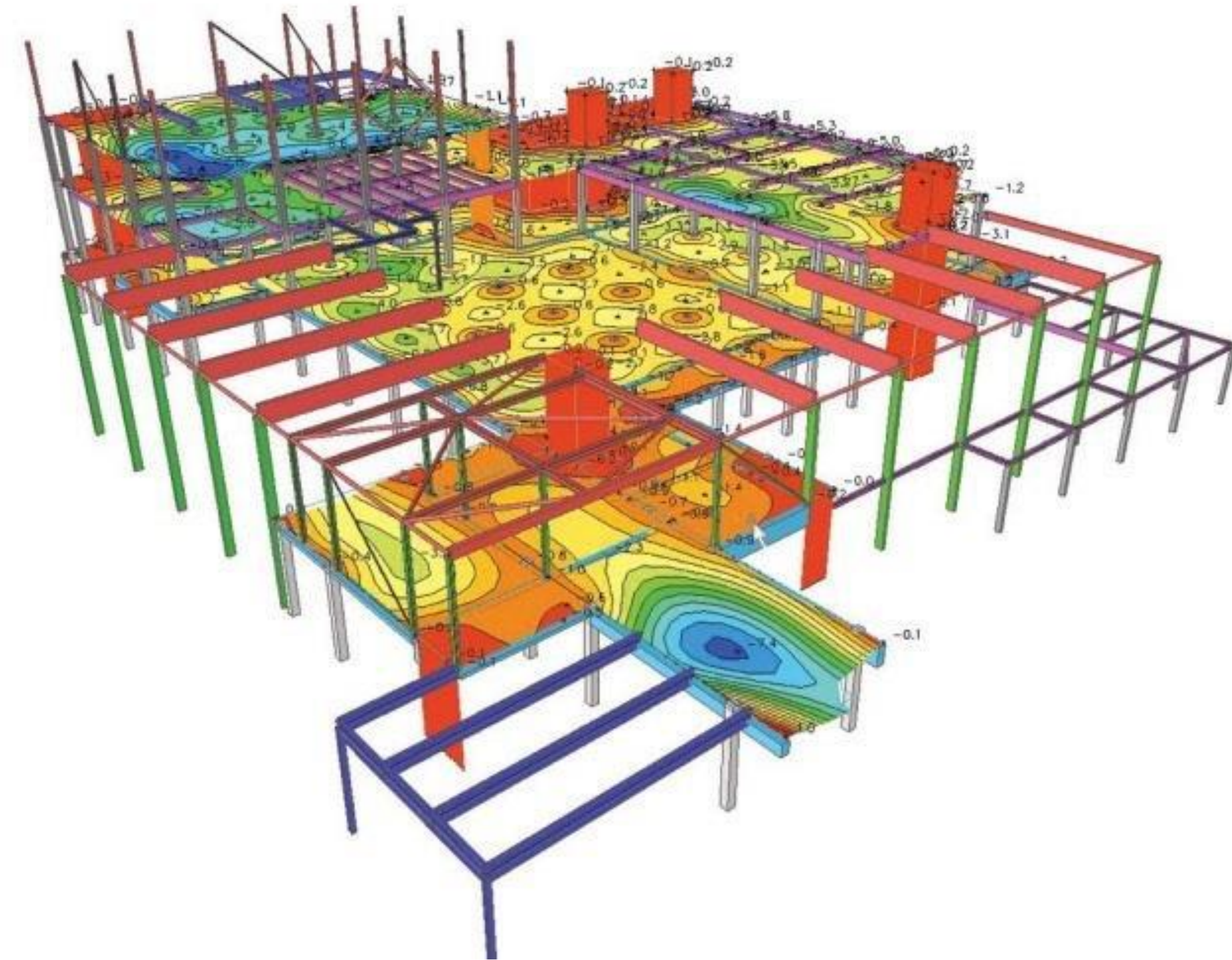
Masonry System

- Stairs: 8" masonry walls with #5@24" o.c. vertical reinf
- Elevators: 12" walls with #5@24" o.c. vertical reinforcement
- Capable of carrying all lateral load without steel moment frames



Moving Away From Simplicity

- Previously with limited software, slower computers, or no software and computers, we simplified reality with conservative “approximations”
- Large difference between all "lateral" and "gravity/lateral" member modeling
- How can members be ignored from lateral system?



Understand Software Capability/Limitations of Analysis

- Beam, Column, and Wall Properties
- Diaphragm Properties
- Diaphragm connection to lateral support system





Beams and Columns



Beams and Columns

- Member properties - boundary conditions
 - Strong axis - pinned or moment connected
 - Maybe semi-rigid?
 - Weak axis and torsion being checked?
- Concrete
 - Does FEA consider cracked sections?



Beams and Columns

- Member properties - boundary conditions
 - End zone
 - Rigid end offsets
 - Pinned, fixed or spring support?

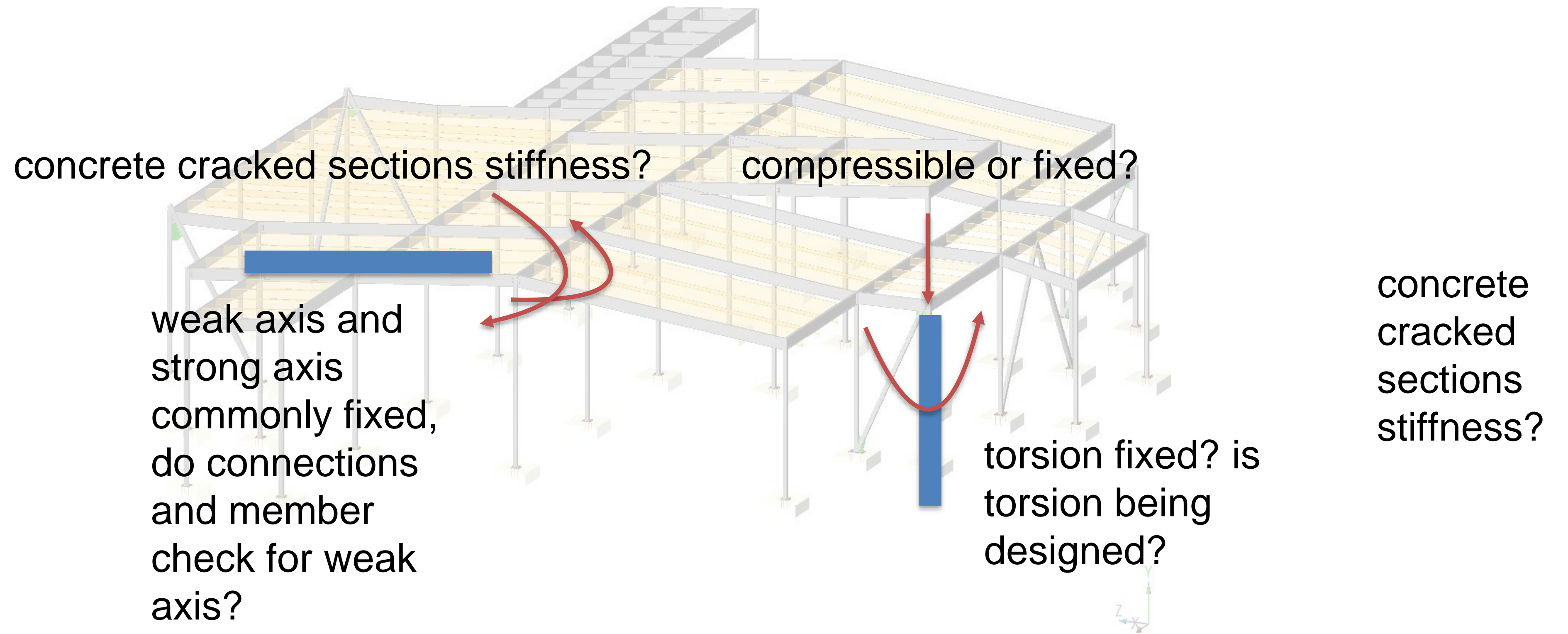


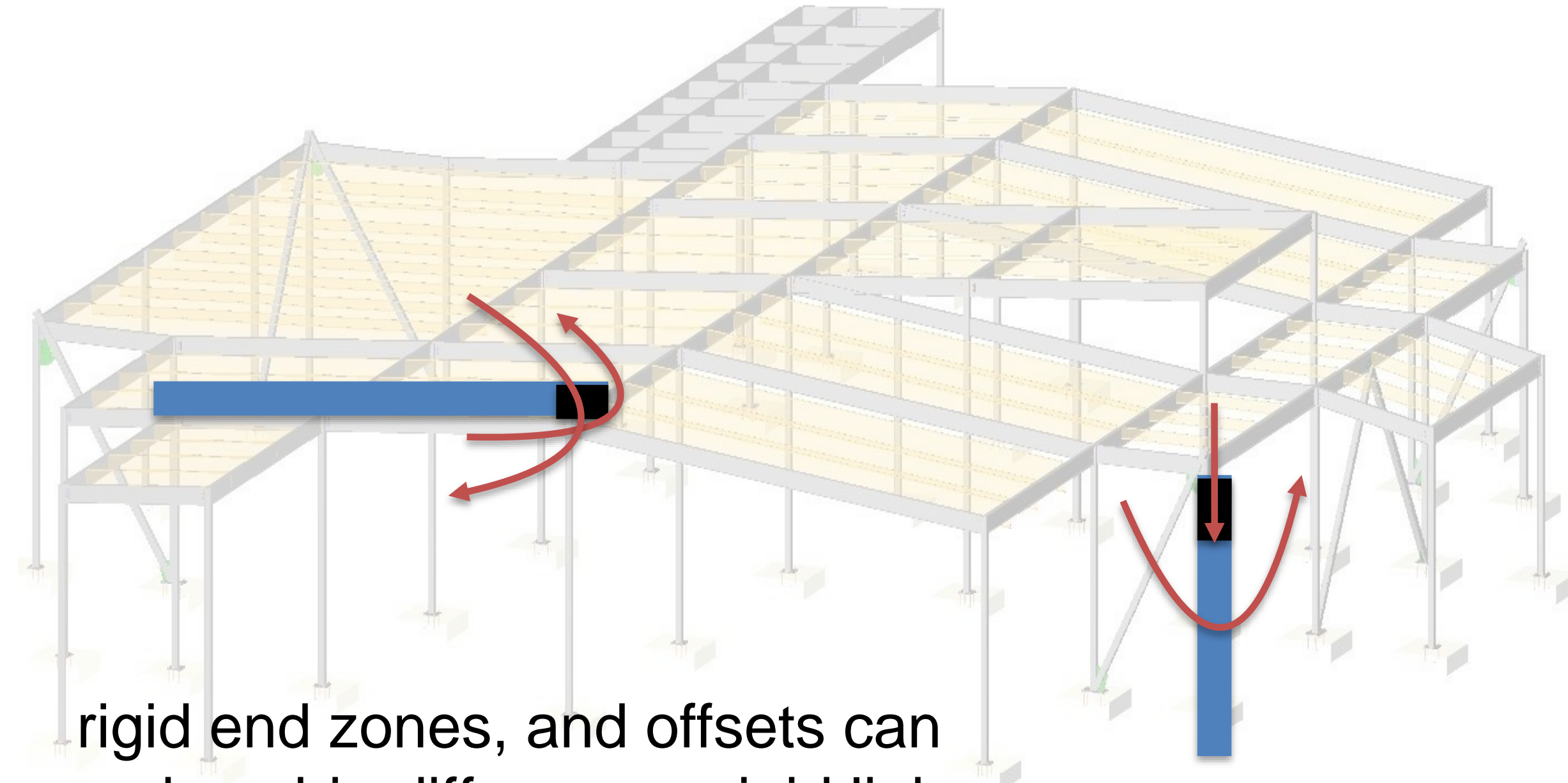
Beams and Columns

- Member properties - type of finite element
 - Wide concrete beam
 - When should it be considered a plate instead of line element (4 node instead of 2)?
 - Large “deep column” or “short wall”
 - Remember, “columns” are 1-D finite elements that connect to plates at a single point



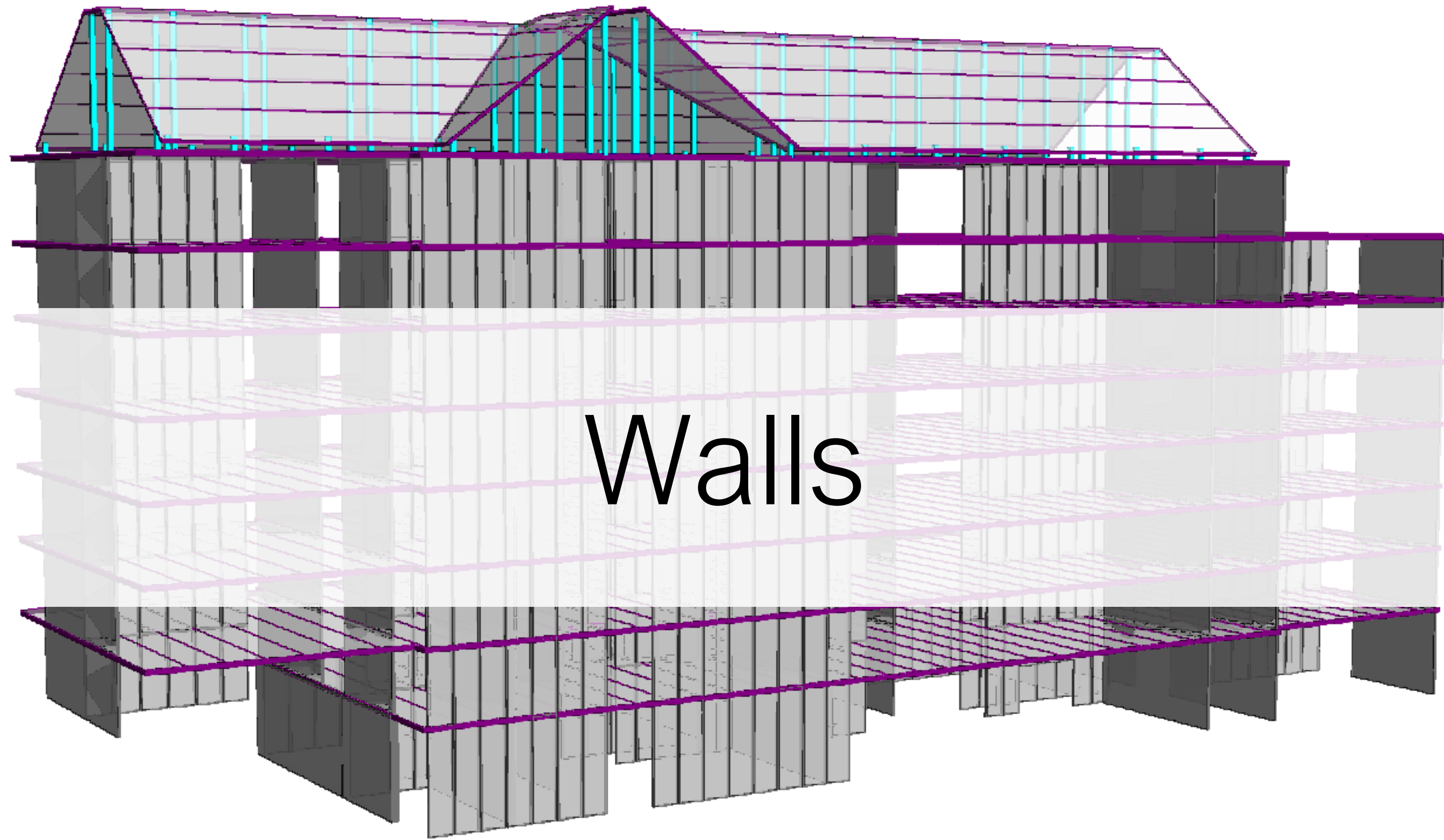
Beams and Columns





rigid end zones, and offsets can make a big difference - rigid link between the end of the member and end node





Walls



Walls

- Wall properties, boundary conditions
 - Is there weak axis bending? torsional?
 - Horizontal and vertical bending? both being checked?
 - Does FEA consider cracked sections
 - Wall node releases

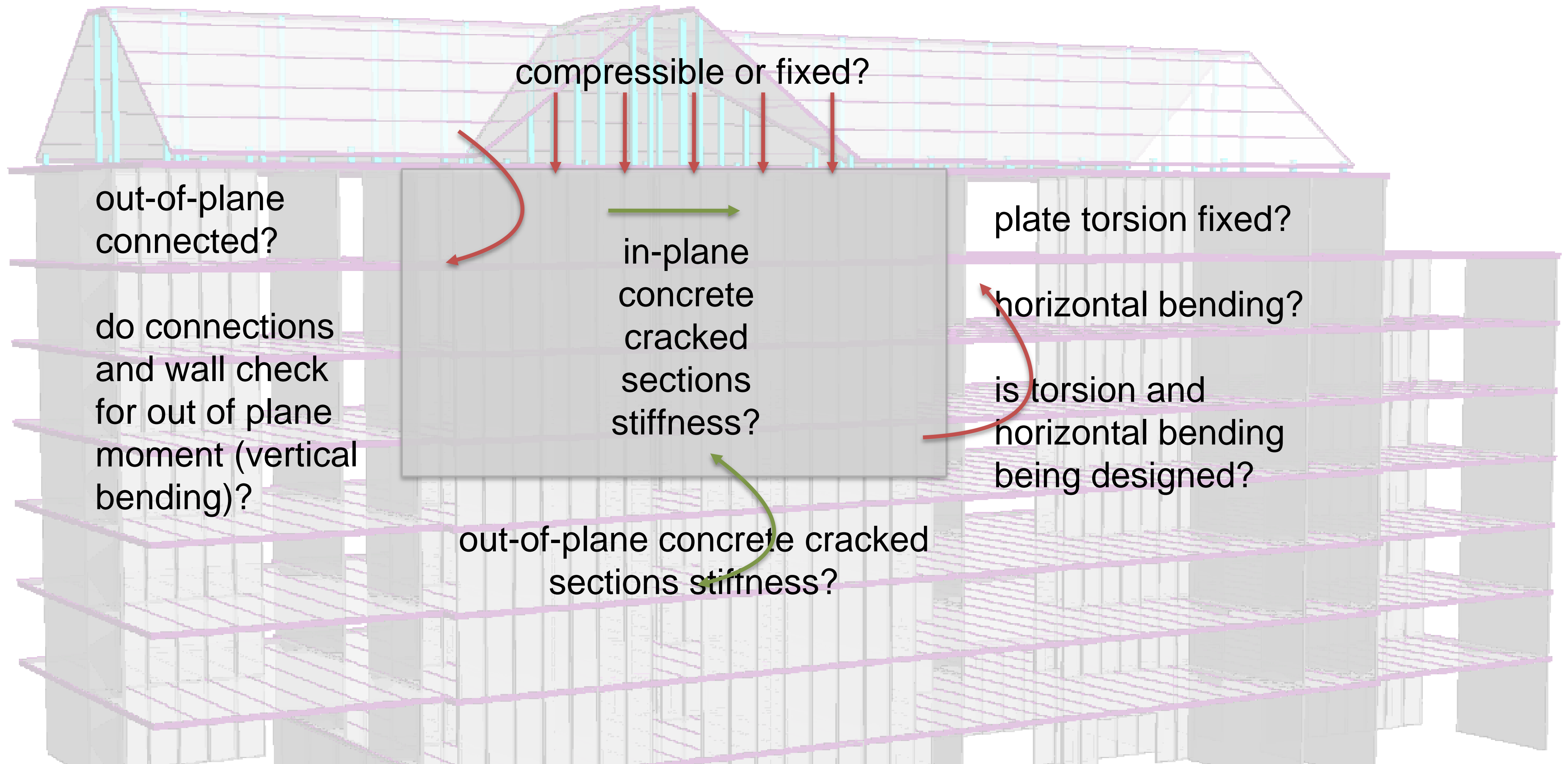


Walls

- Wall modeling
 - Masonry wall stiffness based on partial or full grout
 - “True” long walls vs. short segments
 - Gap or no gap
- Openings



Walls

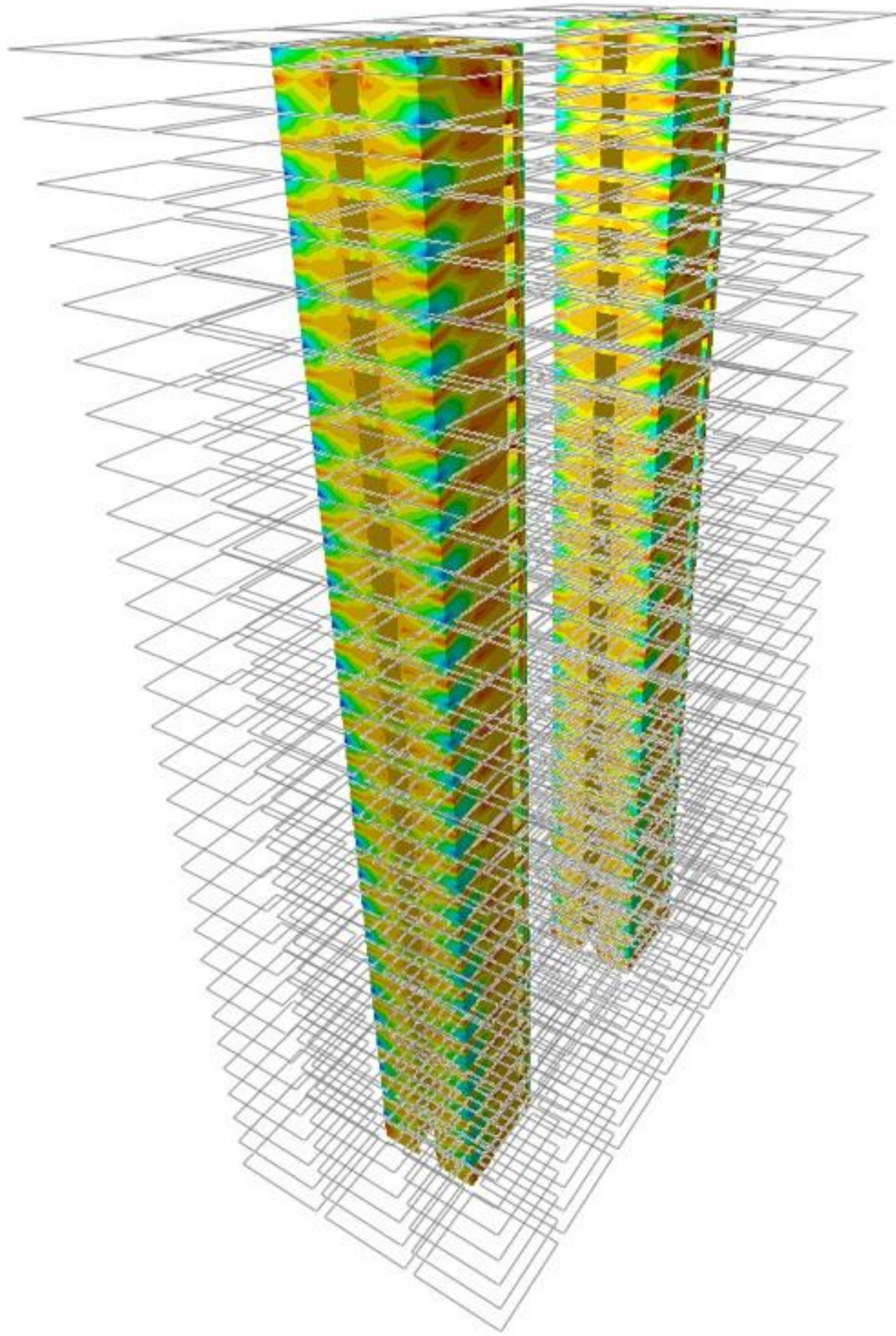


Effective Stiffness for Modeling Reinforced Concrete Structures

By [John-Michael Wong, Ph.D., S.E.](#), [Angie Sommer, S.E.](#), [Katy Briggs, S.E.](#) and [Cenk Ergin, P.E.](#)

STRUCTURE MAGAZINE in [Articles](#), [Structural Analysis](#), January 2017

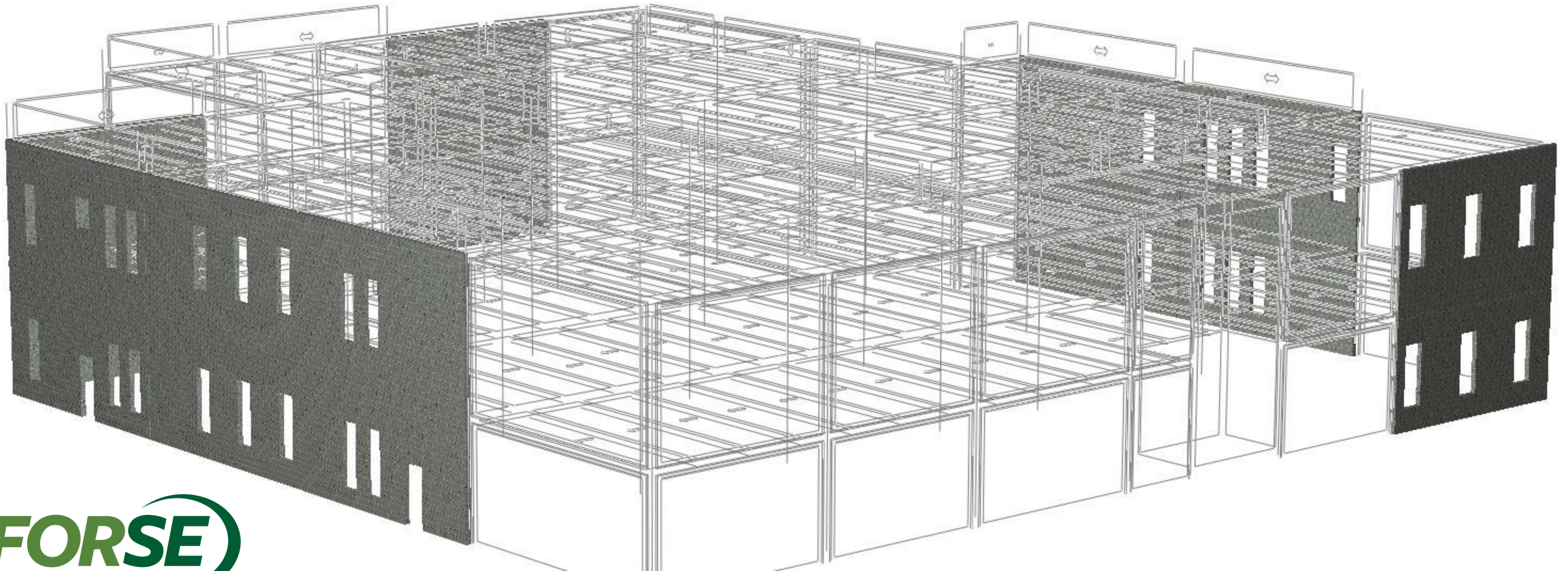
	Elements	Property Modifier for Modeling Elements														
		ACI 318-11 10.10.4.1 ACI 318-14 6.6.3.1.1	ASCE 41-13 Table 10-5	PEER TBI Guidelines Service Level	LATBSDC MCE-Level Non Linear Models (2014)	LATBSDC Servicability & Wind (2014)	FEMA 356 Table 6-5	NZS 3101: Part 2:2008 Ultimate Limit State ($f_y=300\text{Mpa}$)	NZS 3101: Part 2:2008 Servicability Limit State ($\mu=3$) (Note 3)	CSA A23.3-14	EuroCode	TS 500-2000	Paulay & Priestley (1992)	Priestly, Calvi & Kowalsky (2007)		
Beams	Conventional Beams ($L/H > 4$)	0.35I _g	0.30I _g	0.50I _g	0.35I _g	0.70I _g	0.50I _g	0.40I _g (rectangular) 0.35I _g (T and L beams)	0.70I _g (rectangular) 0.60I _g (T and L beams)	0.35I _g	0.50I _g	0.40I _g	0.40I _g	0.17I _g -0.44I _g		
	Prestressed Beams ($L/H > 4$)	n/a	1.00I _g	1.00I _g	n/a	n/a	1.00I _g	n/a	n/a				n/a	n/a	n/a	n/a
	Coupling Beams ($L/H \leq 4$)		n/a	n/a	0.20I _g	0.30I _g	n/a	0.60I _g (diagonally reinforced)	0.75I _g				(9)	n/a		
Columns	Columns - $P_u \geq 0.5A_g f_c$	0.70I _g	0.70I _g	0.50I _g	0.70I _g	0.90I _g	0.70I _g	0.80I _g	1.00I _g	0.70I _g	0.50I _g	0.80I _g (Note 6)	0.80I _g	0.12I _g -0.86I _g		
	Columns - $P_u \leq 0.3A_g f_c$						0.50I _g	0.70I _g	0.90I _g				0.50I _g		0.55I _g	0.80I _g
	Columns - $P_u \leq 0.1A_g f_c$		n/a	n/a	n/a	0.50I _g									0.40I _g	0.70I _g
	Columns - tension						0.30I _g	n/a	n/a				n/a		n/a	n/a
Walls (4)	Walls - uncracked	0.70I _g	n/a	0.75I _g	n/a	n/a	0.80I _g	n/a	n/a	0.7I _g	0.50I _g	n/a	(9)	n/a		
	Walls - cracked	0.35I _g	0.50I _g		1.00E _c (1)	0.75I _g	0.50I _g	0.32I _g -0.48I _g	0.50I _g -0.70I _g	0.35I _g	0.50I _g	0.40I _g - 0.80I _g (Note 6)		0.20I _g -0.30I _g		
	Walls - shear	n/a	0.40E _c A _w (10)	n/a	0.50A _g	1.00A _g	n/a	n/a	n/a	n/a	n/a	n/a	(9)	n/a		
Slabs	Conventional flat plates and flat slabs	0.25I _g	See 10.4.4.2	0.50I _g	0.25I _g	0.50I _g	n/a	n/a	n/a	0.25I _g	0.50I _g	n/a	(9)	n/a		
	Post tensioned flat plates and flat slabs	n/a	See 10.4.4.2									n/a			n/a	
	In-plane Shear	n/a	n/a	n/a	0.25A _g	0.80A _g	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a		
Notes		(5)	(2)	(2)	(2)				(3)				(7)			



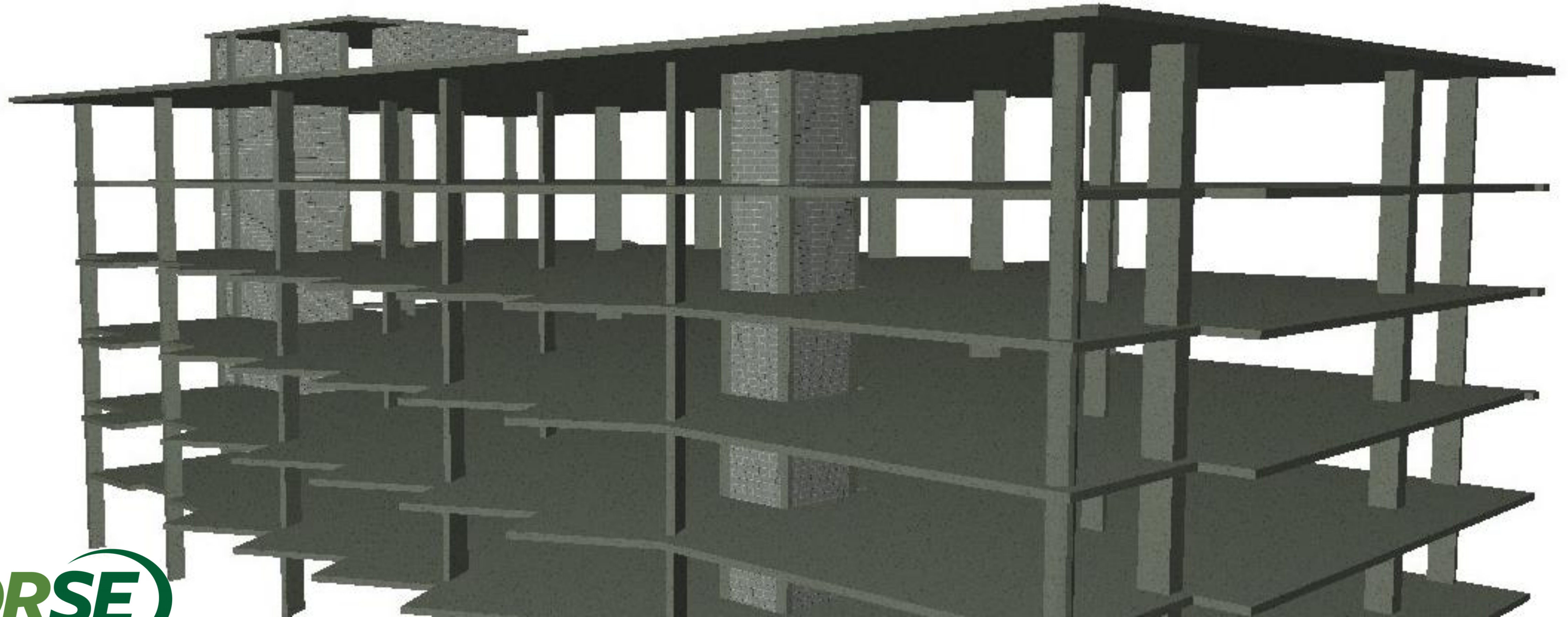
Mixing Materials
... in the same frame



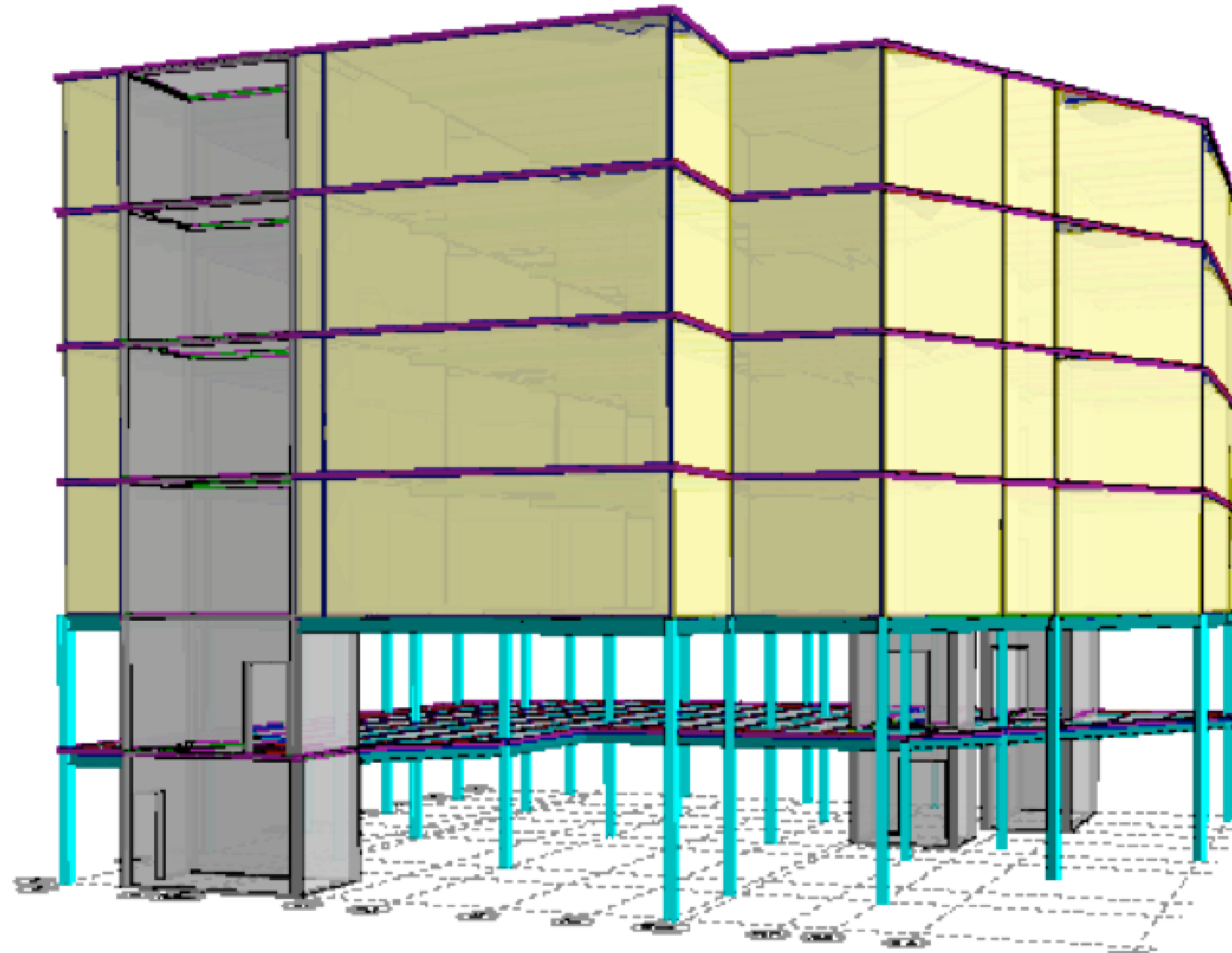
Steel Frames Connected to Perforated Masonry Shear Walls

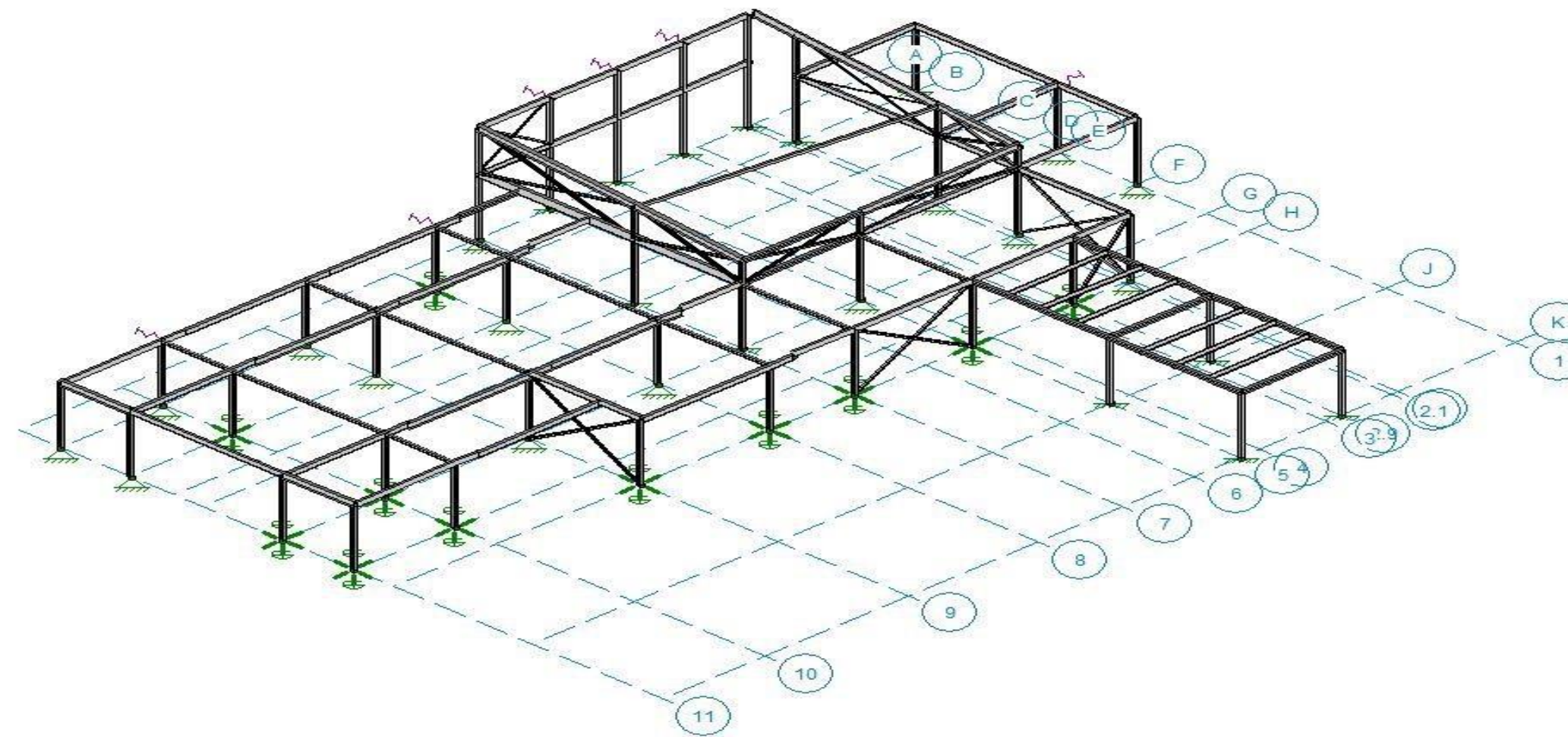


Post-Tensioned Concrete Frame with Masonry Walls



Multiple Material Lateral: Wood - Masonry - Steel





QA/QC for Member Results



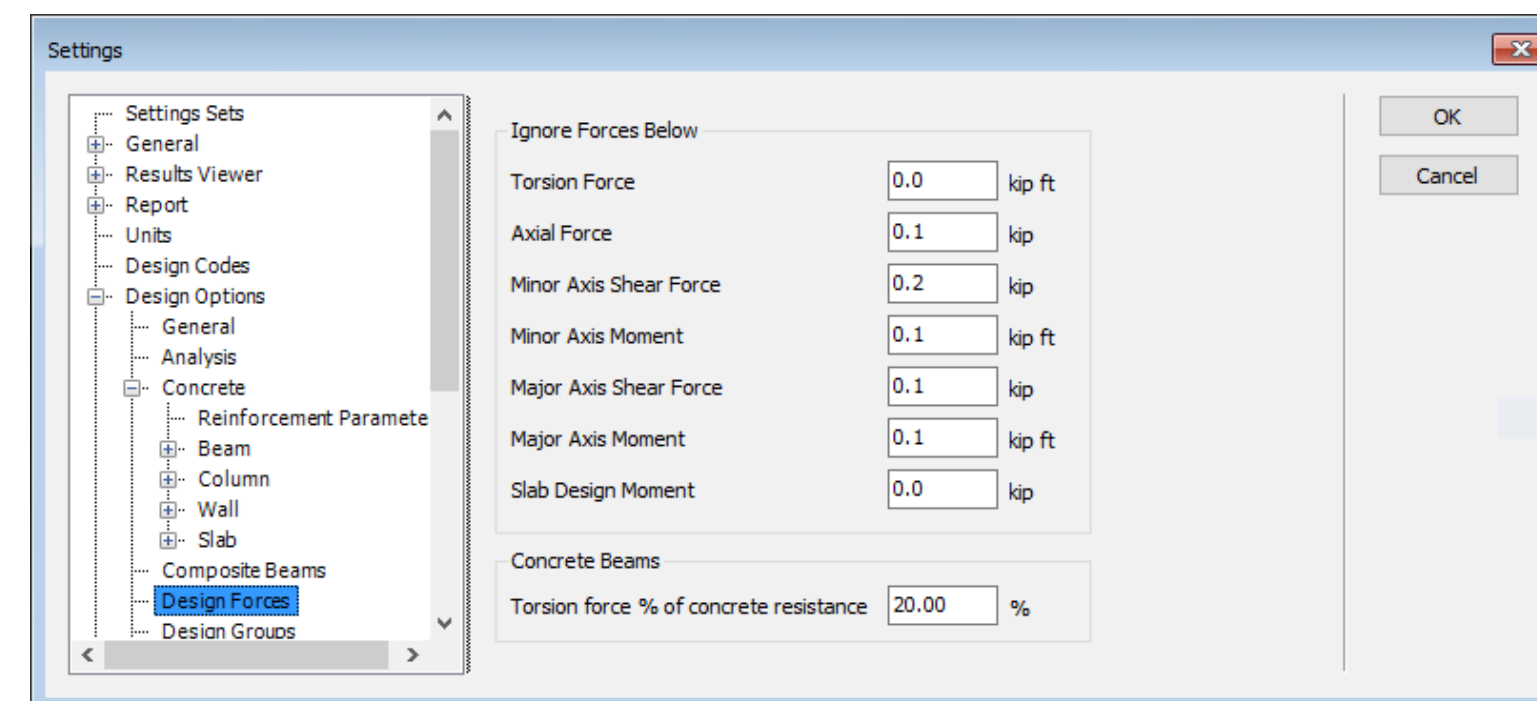
QA/QC

- What are residual forces/stresses?
 - Examples
 - Torsional load in wide flange?
 - Horizontal bending in walls?
 - Axial forces in connections?
 - Diaphragm forces in floors?



QA/QC

- Do you have a way to check for unaccounted for residual forces/stresses?
- Or do you have a means to make sure the magnitude of the load is below a certain threshold to ignore?



QA/QC

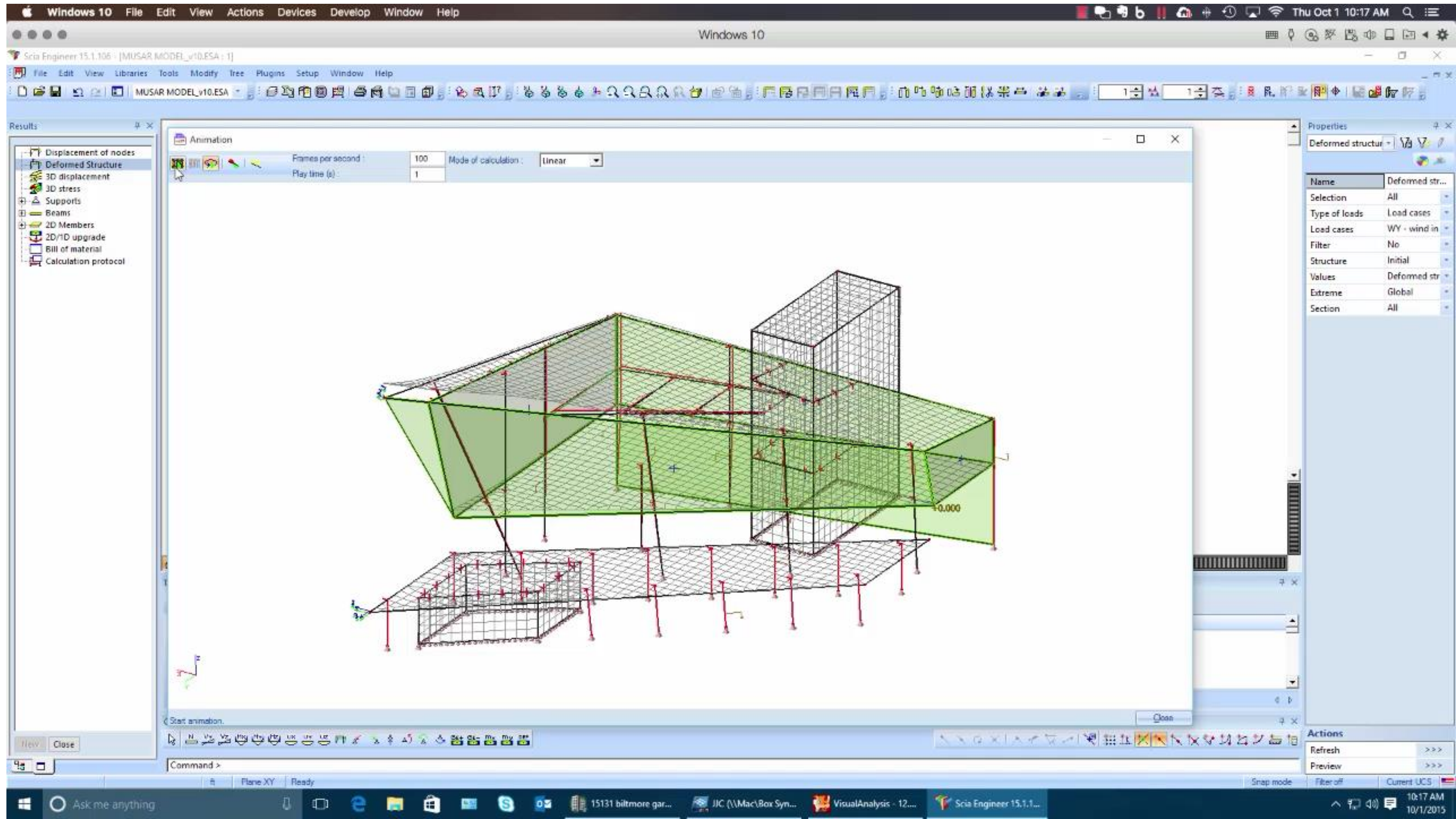
VIEW THE DEFLECTED SHAPE - ANIMATE



QA/QC

- Check drift - inter-story and overall drifts
- Check the animated shapes as well
 - tells a story of the buildings response





Windows 10 File Edit View Actions Devices Develop Window Help Thu Oct 1 12:23 PM

Windows 10 palos_v3 (\\Mac\Home\Downloads\palos_v3.tsmid) - Tekla Structural Designer

File Home Model Edit Load Analyze Design Report Draw Windows Results

1st Order Vibration Strength Factors 1) 1.8 Hz + Reduce Axial Force Service Factors Result Type Scale Settings Support React...

2D Forces Deflections

Fx	Mx	Fxyz	Axial Force	Torsion	X	Dir1	X	Y	M	Fx	Fyz	My	Mdx bottom	X	Deflection	Design Moment	Axial Force	Torsion	Deflection	Support Reaction
Fy	My	Mxyz	Shear Major	Moment Major	Y	Dir2	X Relative	Y Relative	M	Fy	Fyz	Mxy	Mdy top	x bottom	Shear	AsReq	Shear Major	Moment Major	Axial Force	Torsion
Fz	Mz	Total	Shear Minor	Moment Minor	Z	Total	Dir1 Shear	Dir2 Shear	Seismic	Fxy	Mx	Mdx top	Mdy bottom	Y Total	Moment	2D Strip Results	Shear Minor	Moment Minor	Shear	Moment

Wind Model

- Pressure Zones
- Wind Directions
 - +X
- Roof Zones
 - RT 1
 - RT 2
 - RT 3
- Wall Zones
 - +X+Y
 - +Y
 - X+Y
 - +X
 - X-Y
 - Y
 - +X-Y
- Wind Loadcases
 - 5 Wind +X, GCpl -0.18, -C
 - 6 Wind +Y, GCpl -0.18, -C
 - 7 Wind -X, GCpl -0.18, -C

Properties Save... Apply...

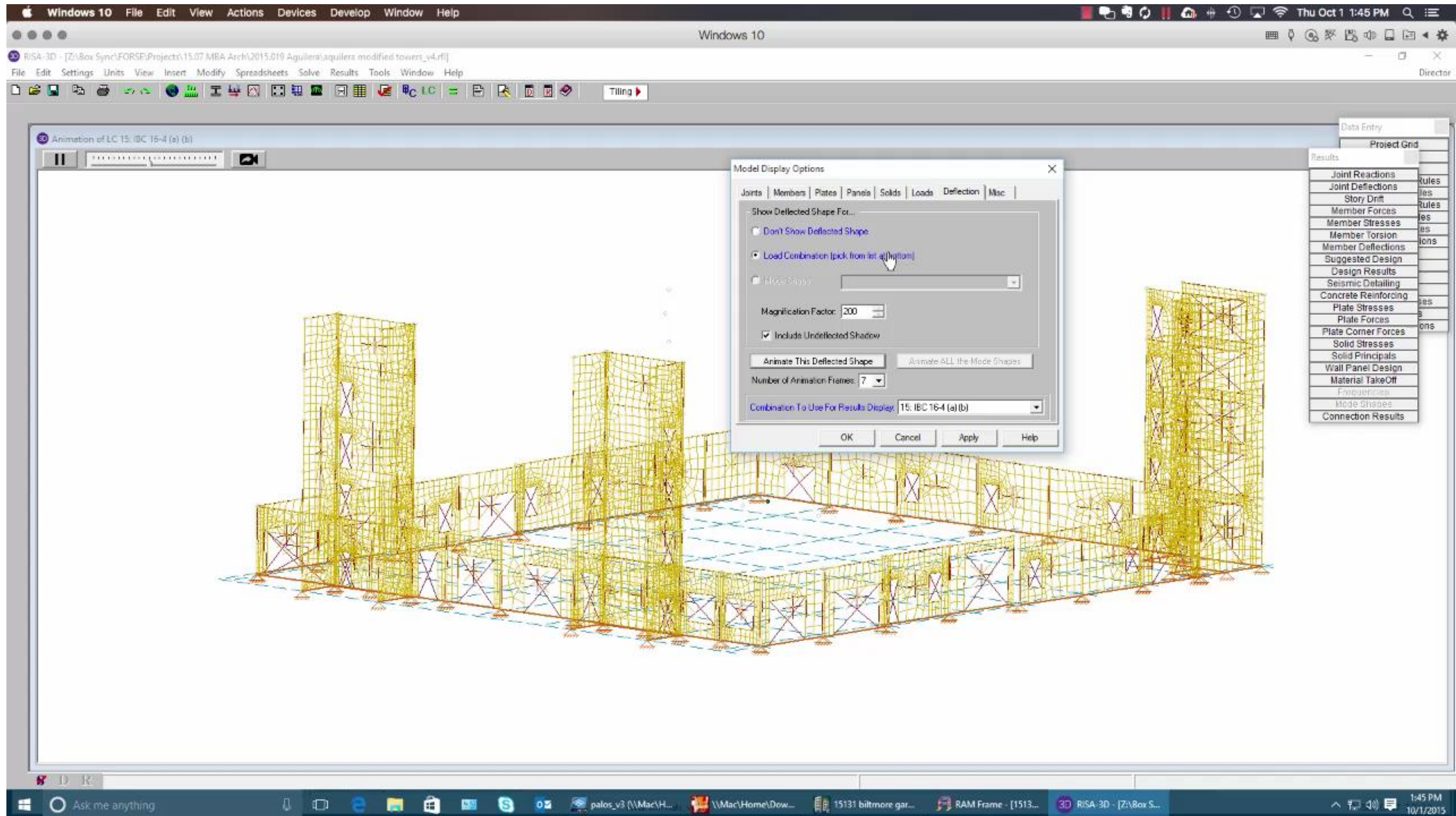
Element Deflection Dir1 min/max=0.0/0.0in

Dir2 Dir1

Solver Model used for 1st Order Vibration

Show Process 140 Effective Seismic Weight US Customary United States (ACI/AISC) REL. POL. 3D

Ask me anything palos_v3 (\\Mac\H... \\Mac\Home\Dow... Snipping Tool eckerd vac_v8 - RA... 12:23 PM 10/1/2015



Project Manager

Results Filter Cut

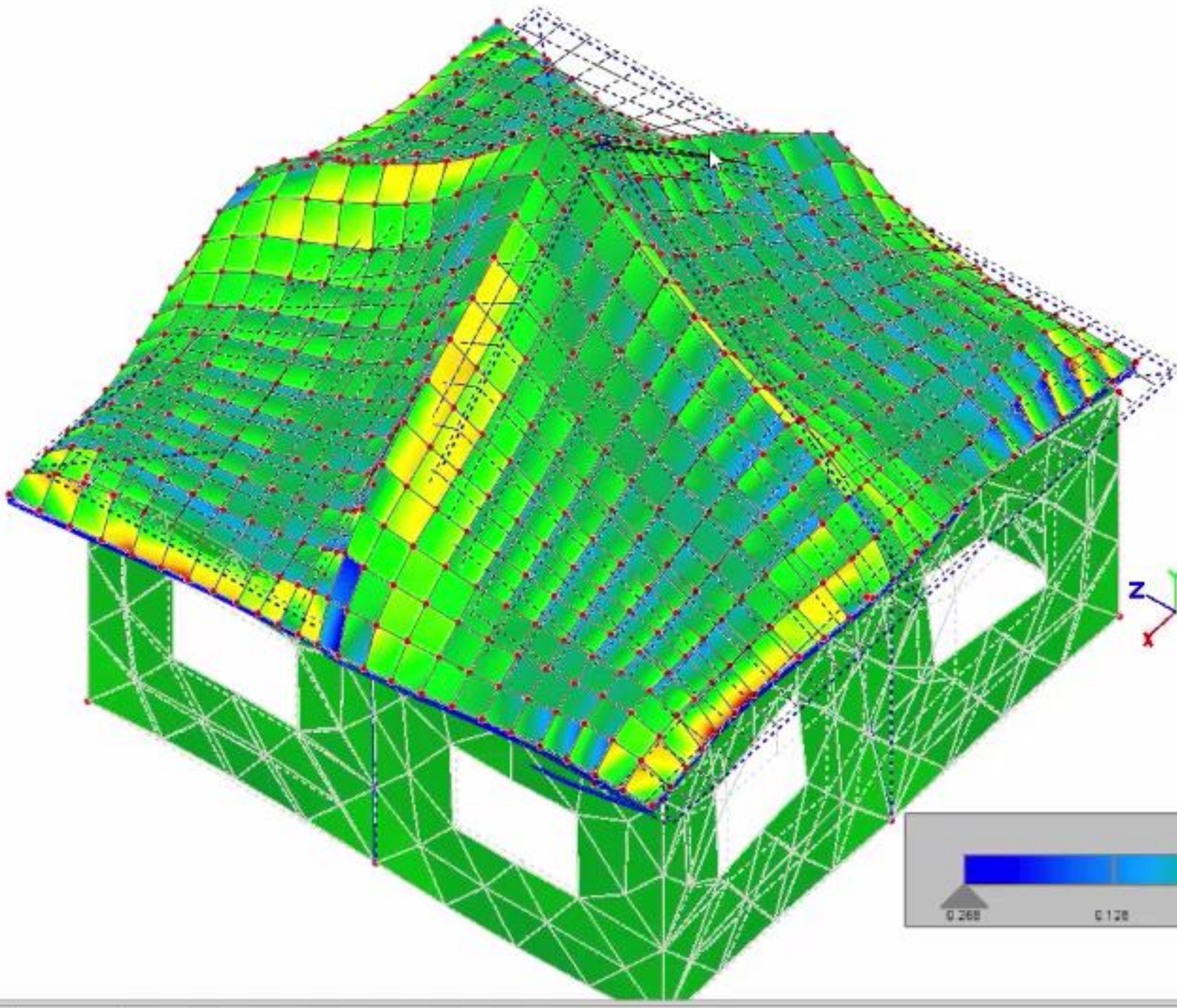
Show or Hide	
Members	<input checked="" type="checkbox"/> Shown
Plates	<input checked="" type="checkbox"/> Shown
Nodes	<input checked="" type="checkbox"/> Shown
Areas	<input checked="" type="checkbox"/> Shown
Results Display	
Analysis Type	First Order
Displaced	<input checked="" type="checkbox"/> Yes
Factor	0.2000
Undisplaced	<input checked="" type="checkbox"/> Yes
Member Results	No results
Graphics	Colors
Plate Results	Sigma x
Plane	Mid-plane
Legend	<input checked="" type="checkbox"/> Yes
Reactions	<input type="checkbox"/> Hidden
Node Displacements	<input type="checkbox"/> Hidden
Area Side Results	None
Member Details	
Plate Details	
Node Details	
Window Options	
Title	<input type="checkbox"/> Hidden
Axes	<input checked="" type="checkbox"/> Shown
Picture View	<input type="checkbox"/> No
Highlight Report	<input type="checkbox"/> No
Fly-by Tips	<input type="checkbox"/> Off

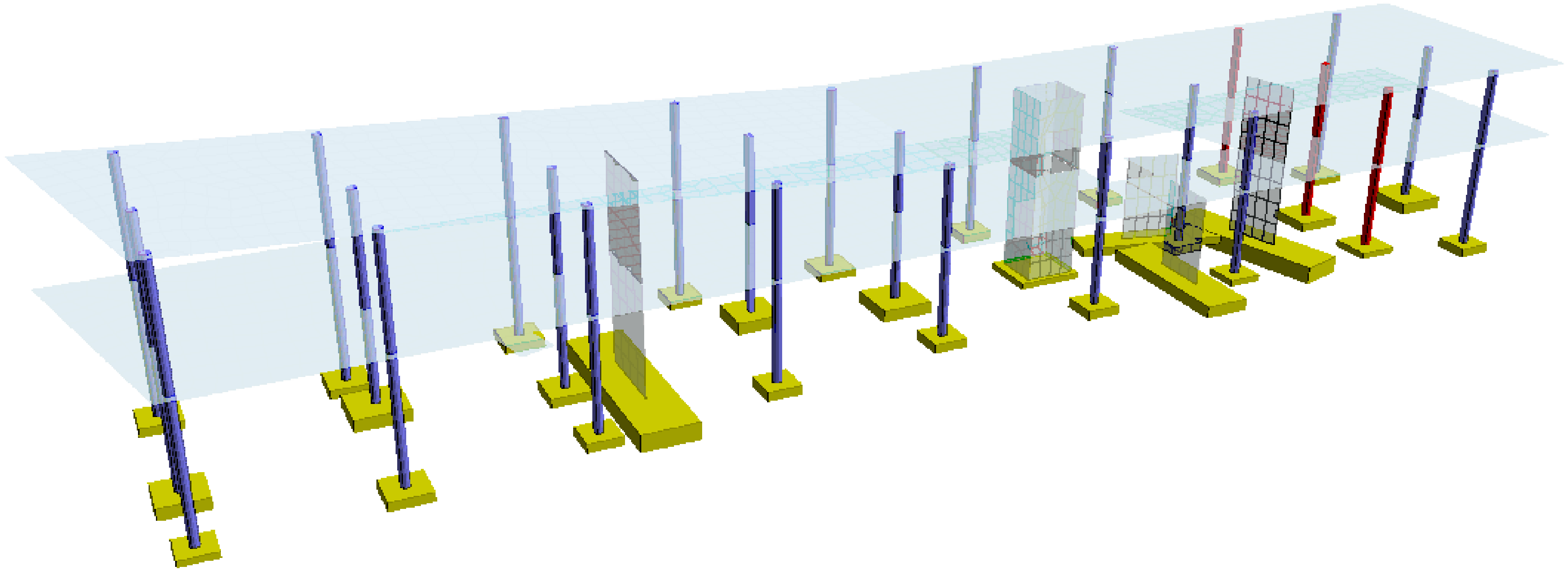
Member Details	
Plate Details	
Node Details	
Window Options	
Title	<input type="checkbox"/> Hidden
Axes	<input checked="" type="checkbox"/> Shown
Picture View	<input type="checkbox"/> No
Highlight Report	<input type="checkbox"/> No
Fly-by Tips	<input type="checkbox"/> Off

Result View Tips
 You can view moment diagrams on a frame using the result type option in the Filter tab.

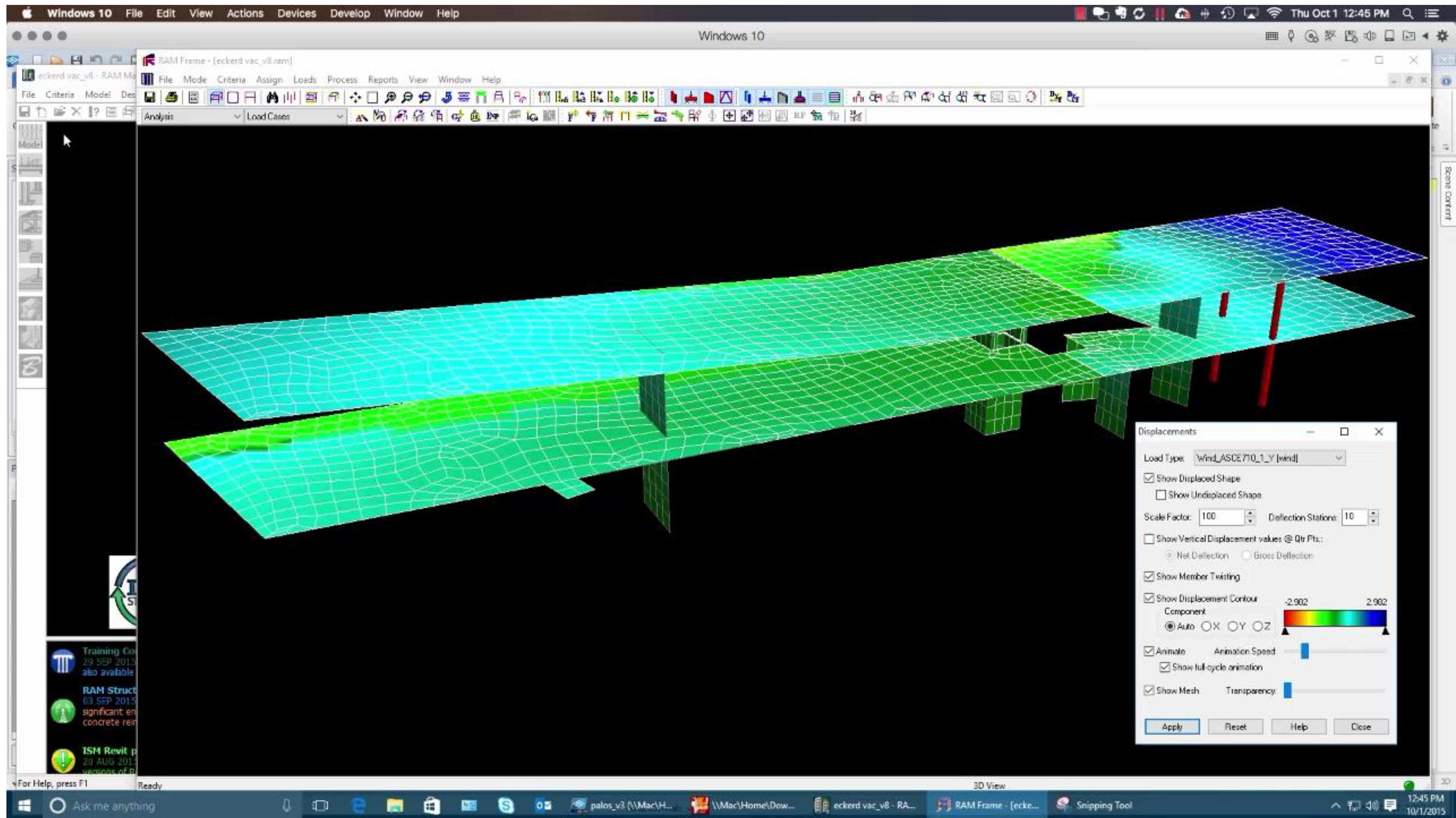
Movie Player - VisualAnalysis Movie Animation

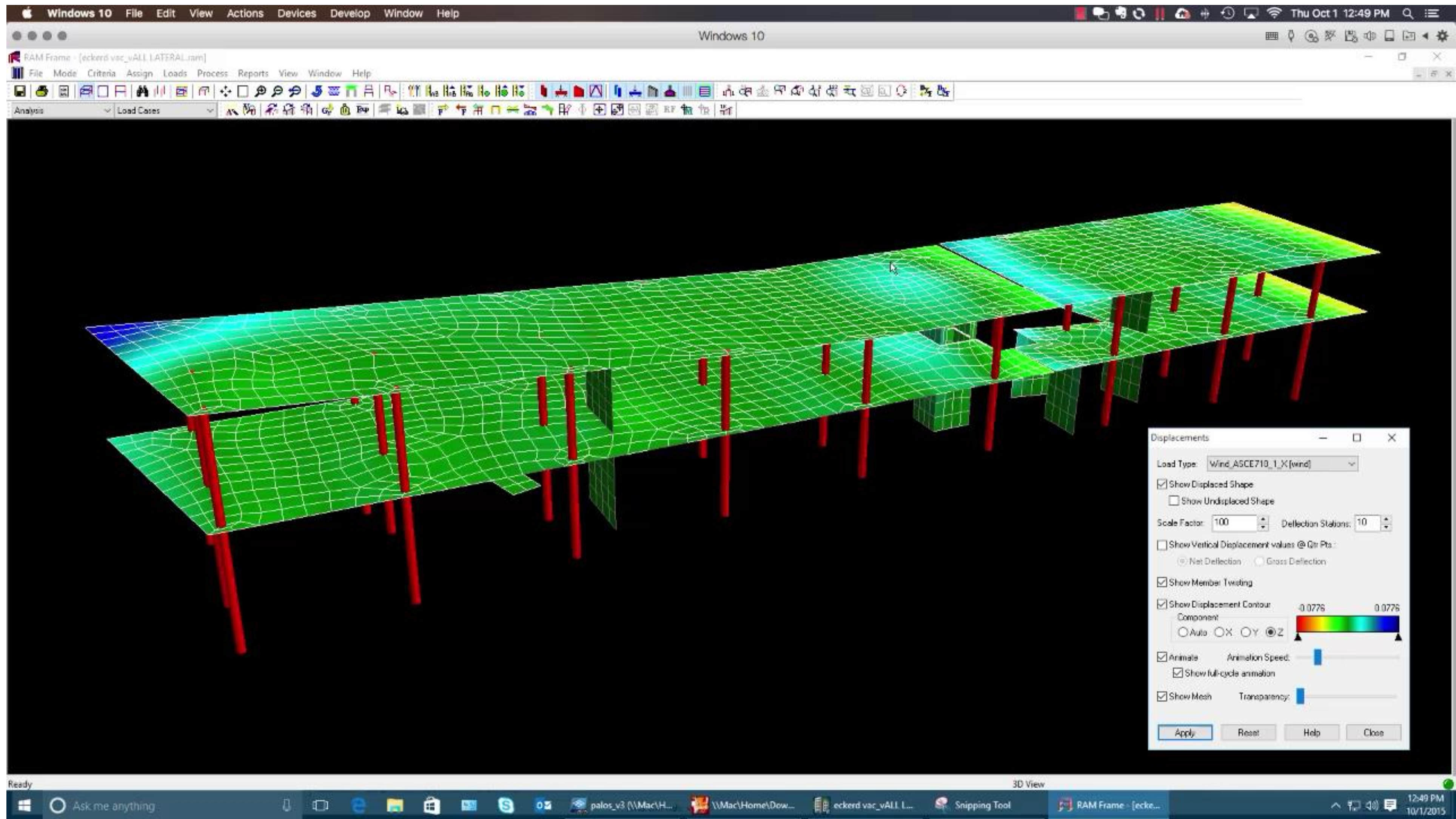
File Movie Help

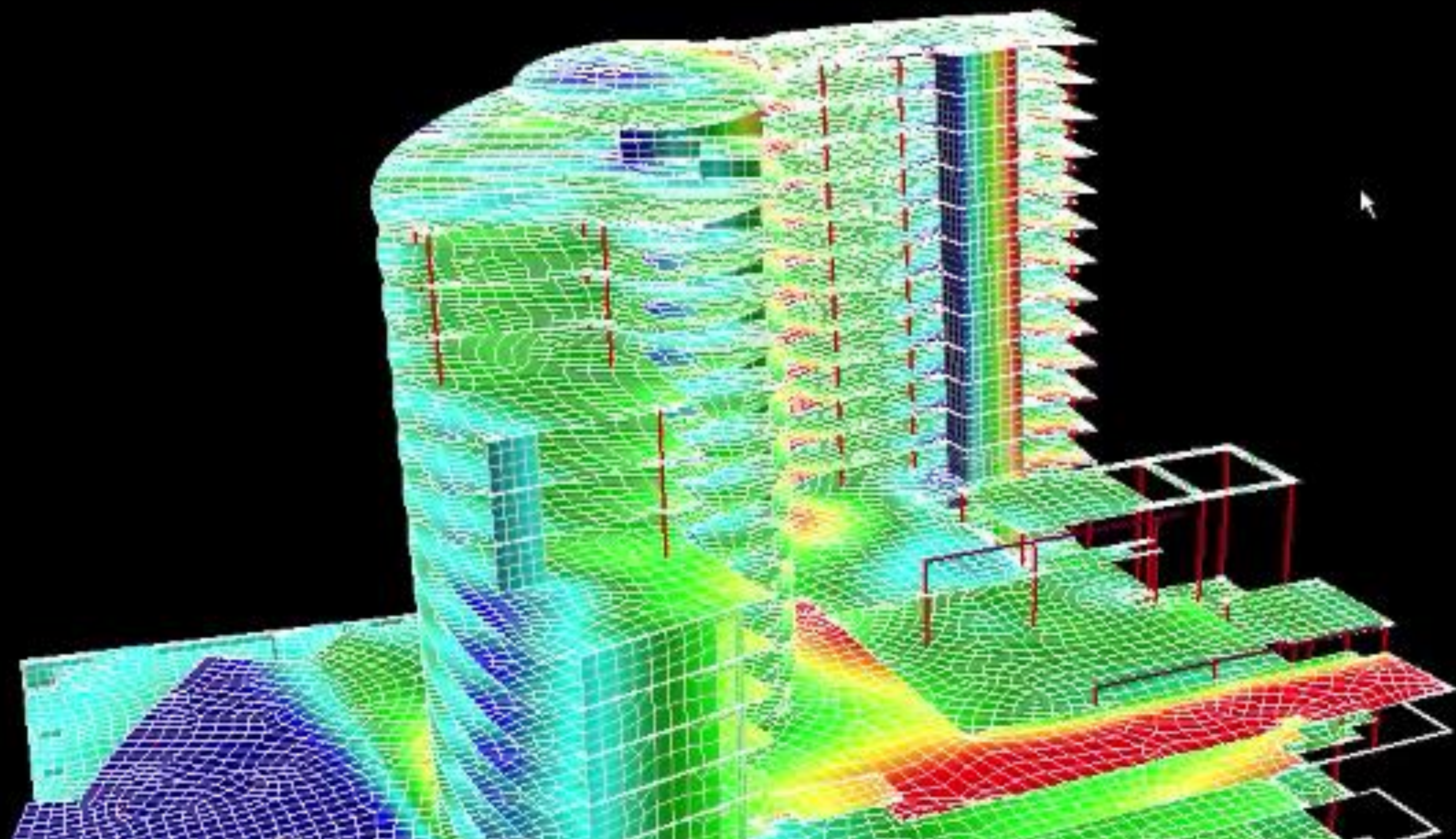




FORSE







Verify design checks

Never go from analysis to design check without validating results first

- Understand software capability/limitations of design checks
 - Note: (obvious) not all programs run the same checks
- start with simple models to understand design checks
 - Note: (obvious) reading the manual is imperative
- be sure design check is comparing the right analysis results against member capabilities



Examples: Floor Vibration Calculations

- structural steel software review
 - all these programs do floor vibration checks
 - do they agree with your hand calcs
 - what to do when things get more complicated



Dynamic Analysis for Steel Floor Vibrations

Simple Software Solution

	A	B	C	D	E	F	G	H	I	J	K	L	M
1													SHEET
2				XXXX	By: XXX			Date: 3/2/2011					
3				0000 0000.00	Checked By:			Date:					
4													
5	Floor Vibrations - Due to Human Activity						AISC - Steel Design Guide Series 11						
6													
7	Chapter 4 - Design for Walking Excitation												
8													
9	Design Criteria												
10	<i>Concrete Slab and Deck Properties</i>												
11	Concrete Thickness												
12	Deck, $t_{DECK} =$		2.00 in		$f_C =$		4000 psi		Loads for Vibration Analysis		DL (slab) =		51.0 psf
13	Concrete, $t_{CONC} =$		3.25 in		$\gamma_{CONC} =$		145.0 pcf		DL (deck) =		2.0 psf		
14	Overall, $t_{TOTAL} =$		5.25 in		$E_{CONC} =$		3492 ksi		DL (mech & misc) =		4.0 psf		
15	Normal Weight Concrete				$E_{CONC} \text{ (dynamic)} =$		1.35 * E_{CONC}		Total DL =		57.0 psf		
16													
17	<i>Steel Section Properties</i>												
18	Steel Beam Section		W18x50		$F_Y =$		50 ksi		LL (for vibration analysis) =		11.0 psf		
19	Steel Girder Section		W24x55		$E_{STEEL} =$		29000 ksi		Modular Ratio, $n = E_{STEEL} / E_{CONC} \text{ (dynamic)}$		6.15		
20													
21													
22													
23													
24													
25													
26													
27													
28													
29													
30													
31													
32													
33													

Dynamic Analysis for Steel Floor Vibrations Simple Software Solution

	A	B	C	D	E	F	G	H	I	J	K	L	M
88	<u>Combined Mode Properties</u>												
89	If L_G is less than B_J , the combined mode is restricted and the system is effectively stiffened.												
90	$\Delta_G = (L_G / B_J) * \Delta_G' = 0.186$ in												
91	Otherwise												
92	$\Delta_G = \Delta_G' =$ N/A												
93													
94	<u>Floor Fundamental Frequency</u>												
95	$f_N = 0.18 * \sqrt{(g / (\Delta_J + \Delta_G))} =$ 3.49 Hz												
96													
97	<u>Effective Panel Mode Panel Weight</u>												
98	$W = \Delta_J / (\Delta_J + \Delta_G) * W_J + \Delta_G / (\Delta_G + \Delta_J) * W_G =$ 124.0 k												
99	$\Delta_J / (\Delta_J + \Delta_G) * W_J =$ 111.70 k												
100	$\Delta_G / (\Delta_G + \Delta_J) * W_G =$ 12.29 k												
101													
102	Indicate the appropriate occupancy by selecting from the categories below:												
103	Offices, Residences, and Churches with non-structural components and furnishings												
104													
105	<u>Use the following Parameters from Table 4.1</u>												
106	Constant Force, $P_0 =$ 65.0 lb												
107	Damping Ratio, $\beta =$ 0.03												
108	Acceleration Limit, $a_0/g =$ 0.50% of g												
109													
110	<u>Floor Fundamental Natural Frequency</u>												
111	$a_P/g = P_0 * \exp(-0.35 * f_N) / (\beta * W)$												
112													
113	<u>Individual Beam</u>		$a_B/g = 0.41\%$ of g		OK, <0.5% of g								
114													
115	<u>Individual Girder</u>		$a_G/g = 0.28\%$ of g		OK, <0.5% of g								
116													
117	<u>Combined Panel</u>		$a_P/g = 0.52\%$ of g		NG, >0.5% of g								

Dynamic Analysis for Steel Floor Vibrations Floorvibe

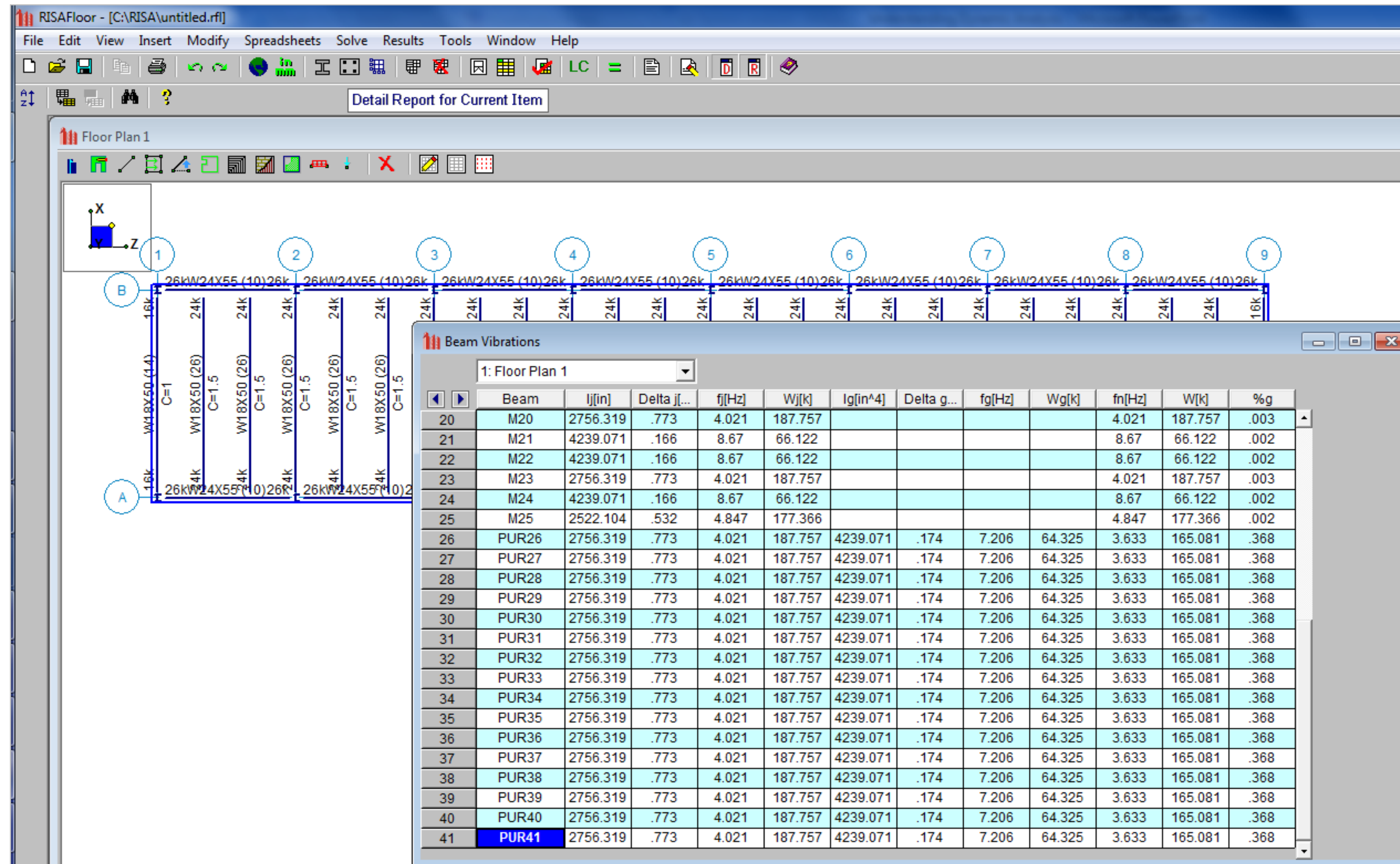
The screenshot displays the FloorVibe software interface. The 'Input Data' section includes fields for Project ID, Project #, Bay ID, By, Criterion (Walking), Occupancy (Paper Office), Acceleration Limit (0.50 % of g), Loadings (Dead: 4.00 psf, Live: 11.00 psf, Collateral: 0.00 psf, Damping Ratio: 0.030), Concrete properties (Total Depth: 5.250 in, fc: 4.00 ksi, Weight: 145 pcf, Deck Height: 2.000 in), Girder Span (30.00 ft), Beam Spans (Left: 0.00 ft, Center: 45.00 ft, Right: 0.00 ft), Girders/Walls (Left: W24X55, Right: W24X55, Beam: W18X50), and Floor Width (240.00 ft) and Floor Length (45.00 ft). A structural diagram shows a floor plan with a central beam and two side girders. The 'Evaluation' section states: 'Evaluation: Combined mode $a_p/g = 0.51\% > 0.50\%$. The system DOES NOT SATISFY THE CRITERION. Beam Frequency 3.84 Hz, Left Girder Frequency 8.19 Hz, Right Girder Frequency 8.19 Hz, Bay Frequency 3.48 Hz. Summary Report/Printout is selected.

can be used as a stand-alone program, or can be used from RAM Steel

Right Girder: W24X55 | Span = 30.00 ft

Dynamic Analysis for Steel Floor Vibrations

RISA Floor



Develop Your Checklist

Design features vary between programs, know what the differences are.

Affiliated Program	TEKLA family	RAM family	RISA family	CSI family	SCIA family	S-FRAME family	Feature Importance	Must Have Feature
Comparison chart - Full Building Program	TEKLA BUILDING DESIGNER	RAM STEEL RAM FRAME	RISA FLOOR RISA 3D	ETABS	SCIA Engineer	S-FRAME		
Modeling								
Easy GUI	✓	✓	○	○	○	○	★★★★★	○
Model in 2D	○	✓	✓	✓	✓	○	★★★☆☆	✓
Model in 3D	○	○	✓	✓	✓	○	★★★★★	○
Integration with REVIT	✓	✓	✓	✓	✓	○	★★★★★	○
Model/Analyze/Design in one program	✓	○	✓	✓	✓	○	★★★☆☆	○
Complex modeling	✓	○	○	○	○	○	★★★★★	○
Loading								
Surface	✓	✓	○	○	○	○	★★★☆☆	✓
Variable	✓	✓	○	○	○	○	★★★★★	○
Line	✓	✓	○	○	○	○	★★★★★	○
Variable	✓	✓	○	○	○	○	★★★☆☆	○
Point	✓	✓	○	○	○	○	★★★★★	✓
Dead load	✓	✓	✓	✓	✓	○	★★★★★	✓
Live load	✓	✓	✓	✓	✓	○	★★★★★	✓
Snow load	✓	✓	○	○	○	○	★★★★★	○
Wind Load (ASCE 7)	✓	✓	○	○	○	○	★★★★★	✓
Wind load (uplift)	✓	✓	○	○	○	○	★★★☆☆	○
Wind load (lateral pressure) based on floor coordinates	○	○	★	★	★	★	★★★★★	○
Wind load (lateral pressure) based on defined exposure areas	○	○	★	★	★	●	★★★★★	✓
Seismic load	○	○	○	○	○	○	★★★★★	✓
Determine Dynamic Properties	○	○	○	○	○	○	★★★☆☆	○
Dependent on building period calculation	○	○	○	○	○	○	★★★★★	○
Custom loads	✓	○	○	○	○	○	★★★★★	○
Custom gravity load combinations	✓	○	✓	✓	✓	○	★★★★★	○
Custom lateral load combinations	✓	✓	✓	✓	✓	○	★★★★★	○
Design - Steel								
Steel Beam	✓	✓	✓	✓	✓	○	★★★★★	✓
Composite beam	✓	✓	✓	✓	○	○	★★★★★	○

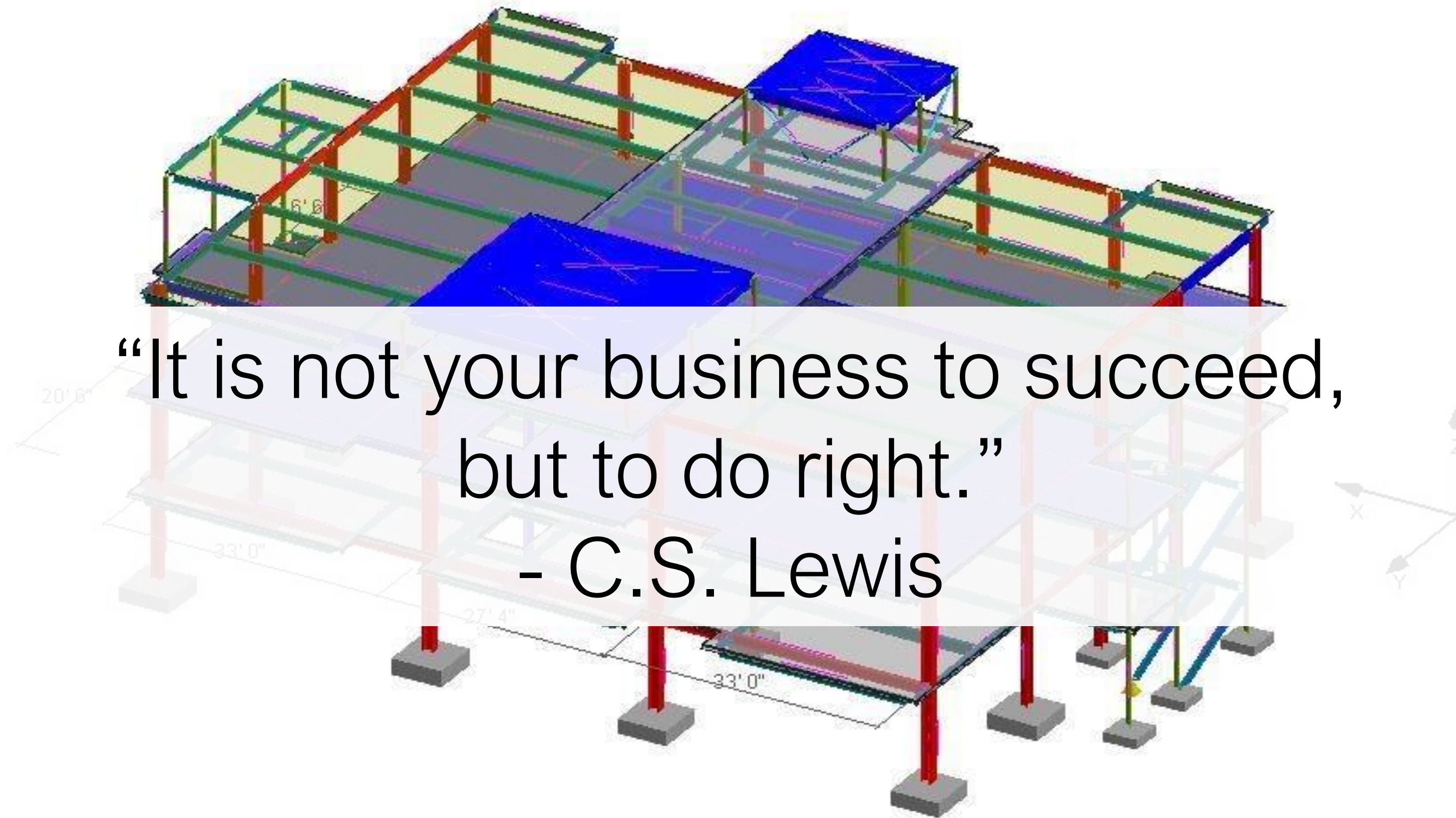


In Conclusion

- Get to know your software, develop means to verify by hand
- Get to know the code, and how it's been implemented in each software you use
- Always know capabilities, and more importantly limitations
- Always, always check with hand calculations

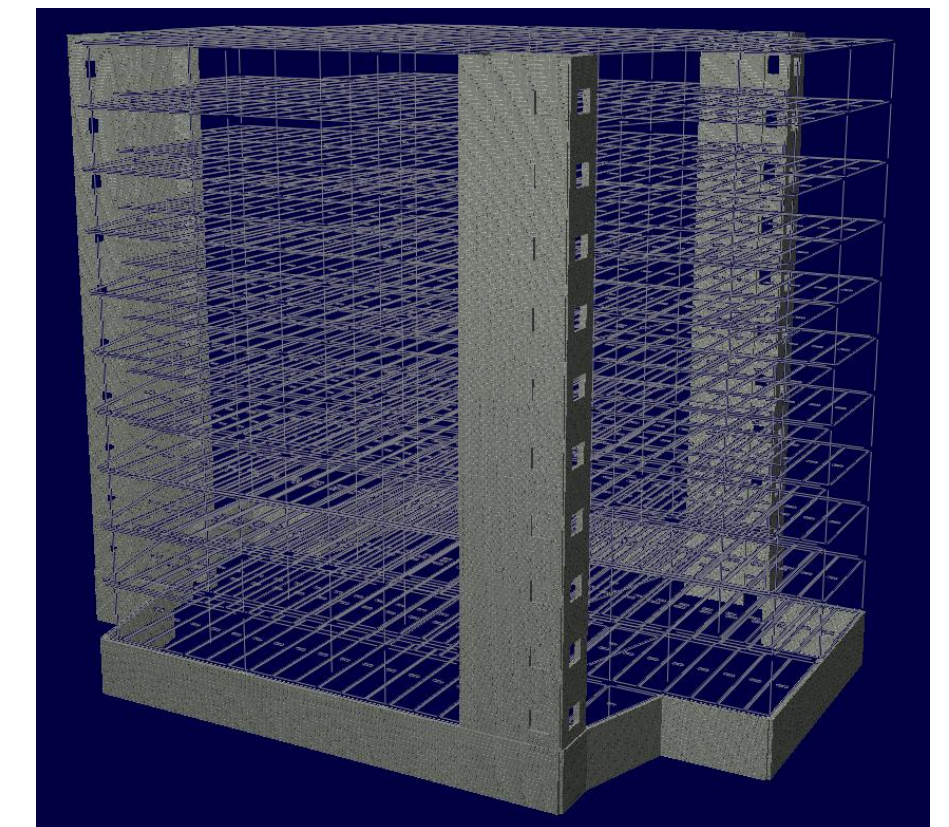
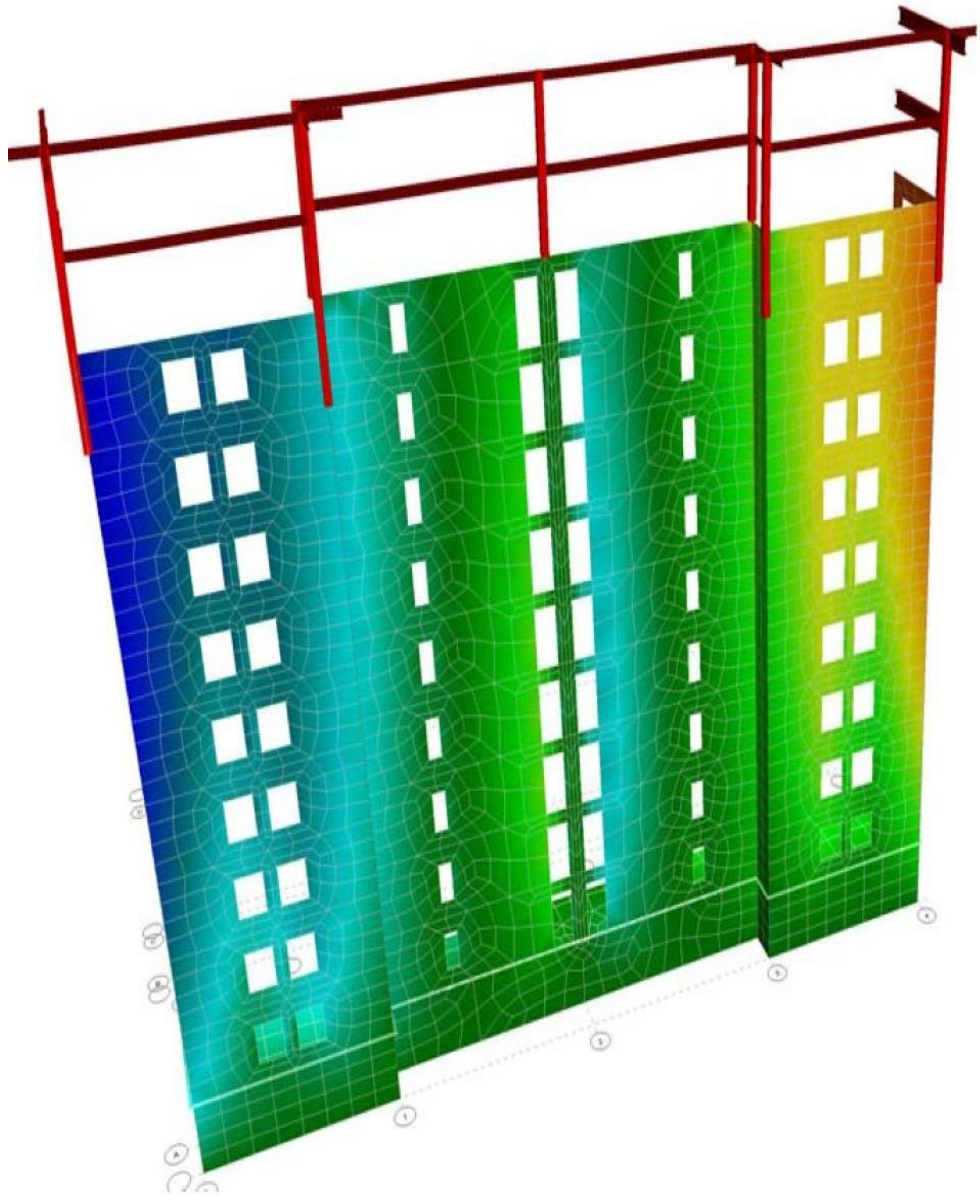
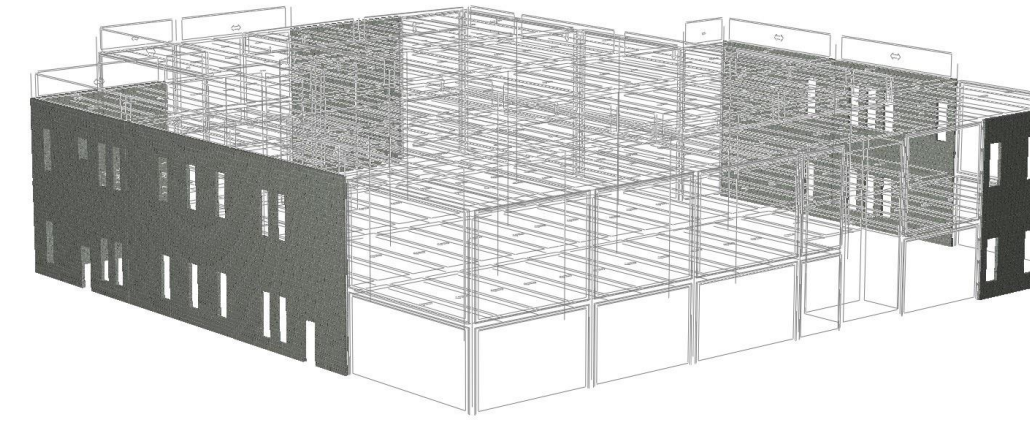
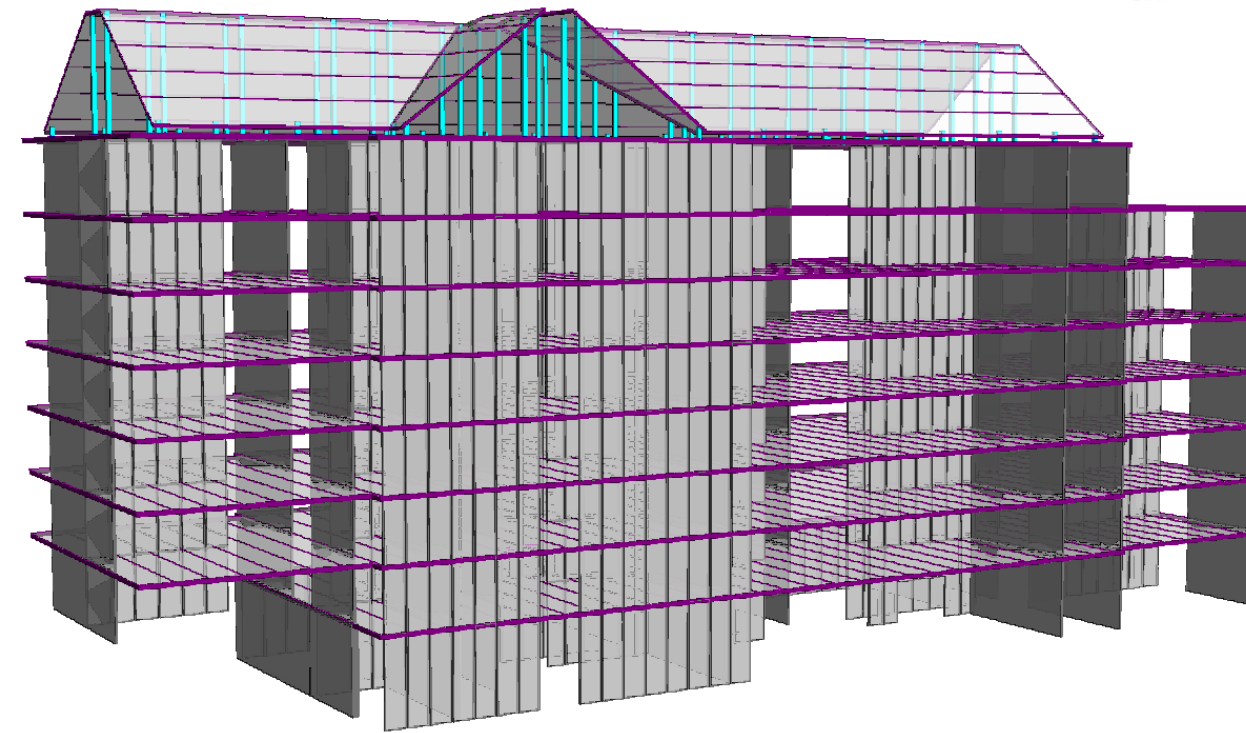
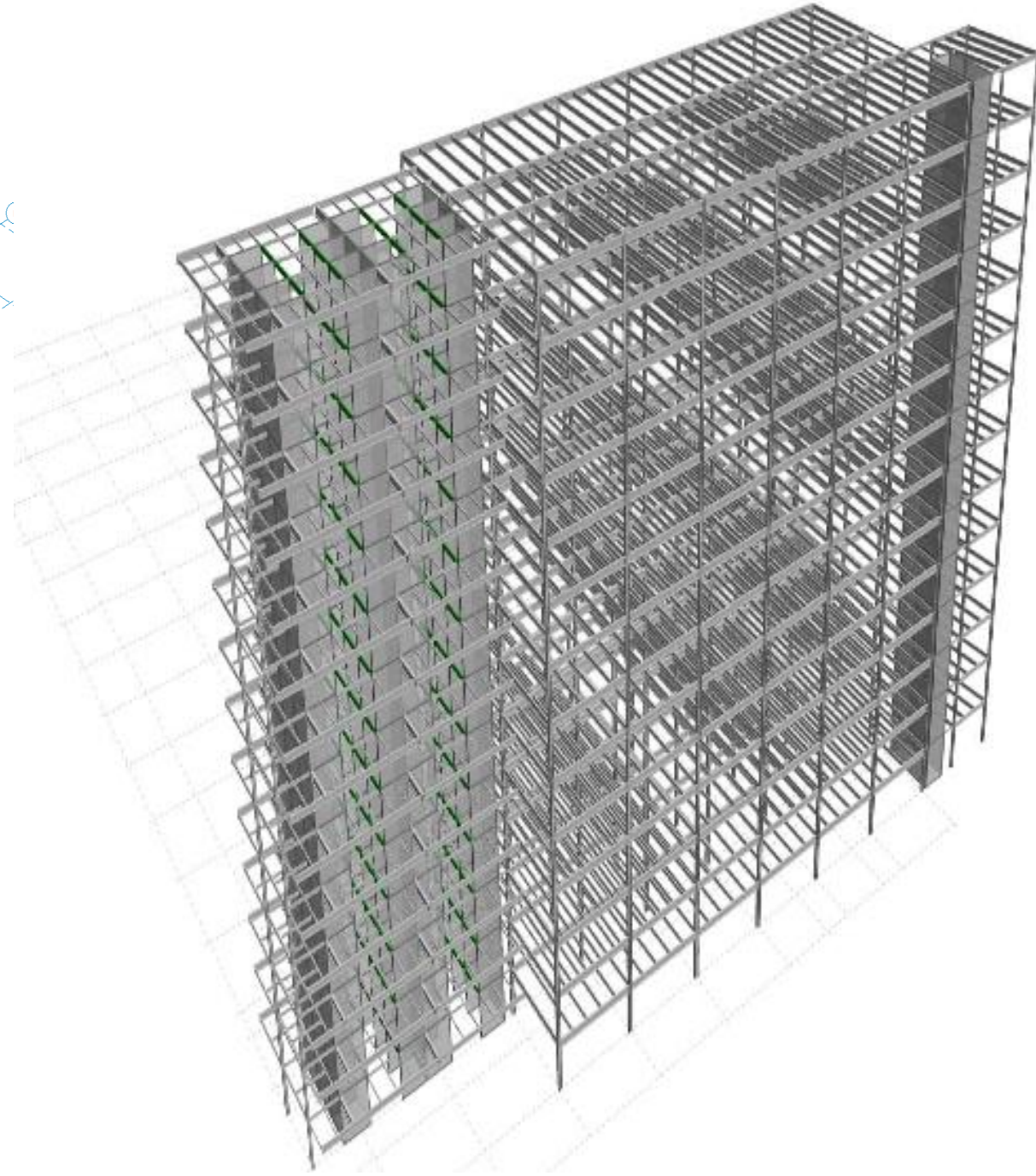
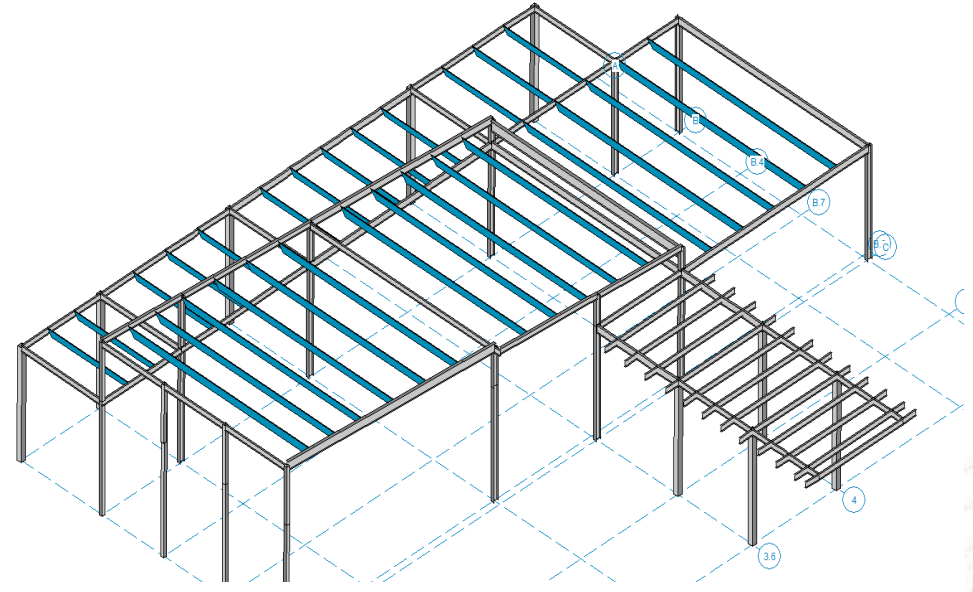
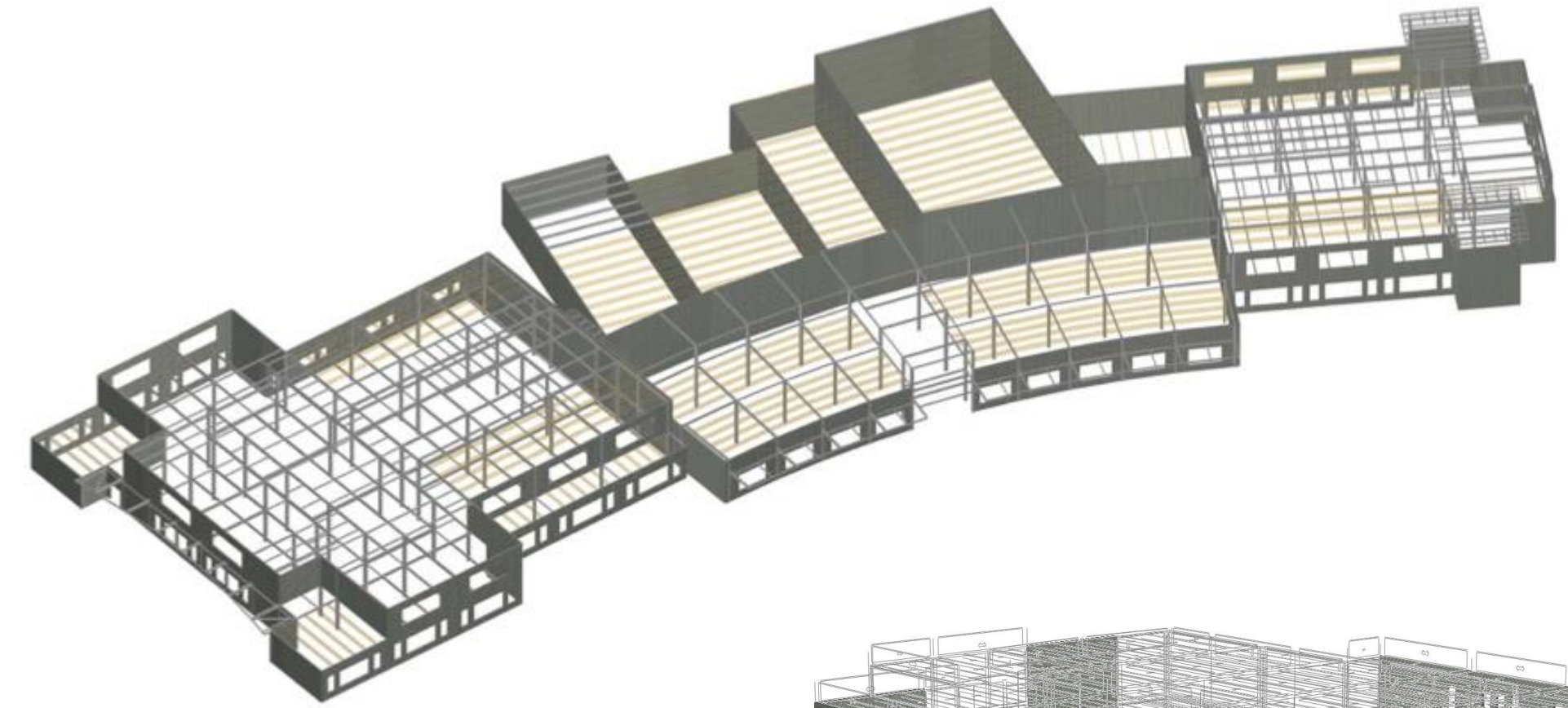
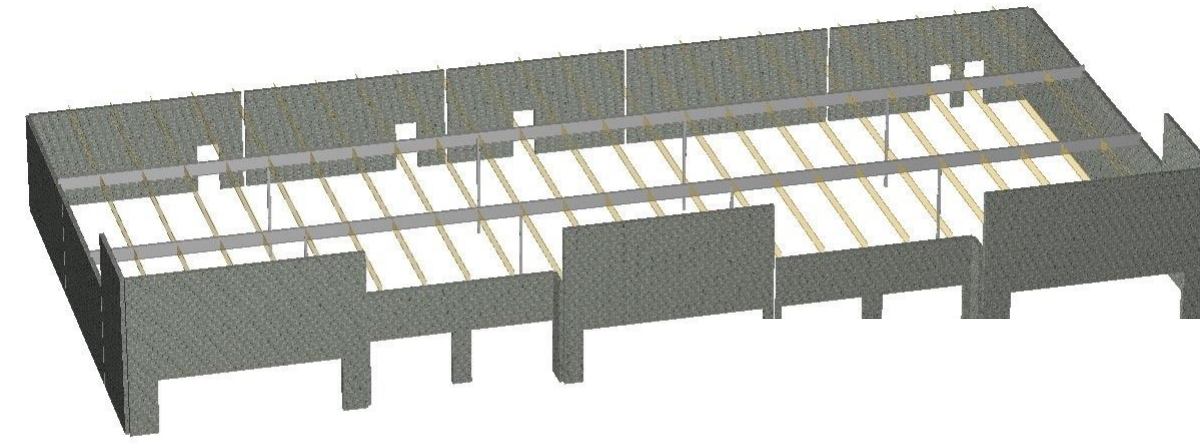
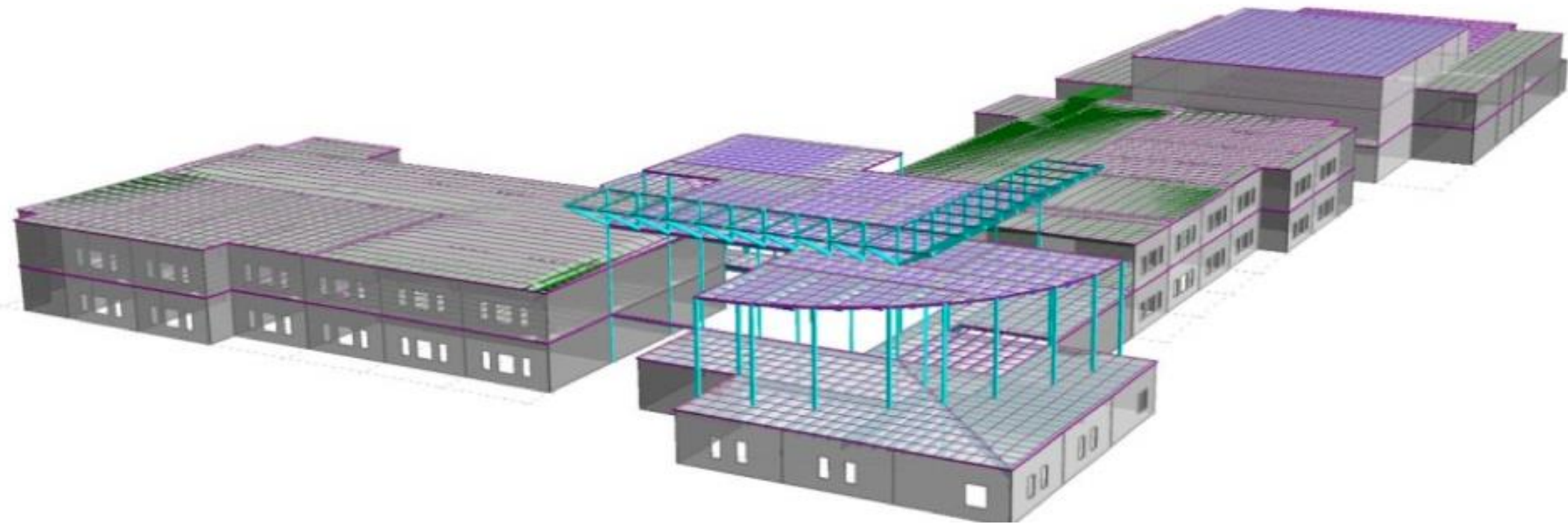
Remember software is a tool, you're the engineer!





“It is not your business to succeed,
but to do right.”
- C.S. Lewis





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